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May 31, 2020 (Revised September 7, 2023)

# ENGINEER'S LETTER REPORT FOR DRAINAGE DESIGN FOR 33 MIDDLEBUSH, LLC TOWN OF WAPPINGER, NEW YORK

The proposed stormwater management facilities have been designed to provide both water quality and quantity controls by detaining, treating, and releasing stormwater runoff at a rate equal to or less than that which existed prior to construction of improvements at the project site.

# 1.0 PROJECT SUMMARY

The parcel currently contains a masonry building and a limited asphalt parking area. The proposed improvements include reconstruction of a building within the same footprint, asphalt and gravel parking/truck maneuvering area expansion, sidewalk construction, landscaping and utility upgrades. The parcel is currently served by a private water supply well and an underground sanitary sewage disposal system.

The project will involve the removal of existing pervious wooded and grass surfaces for the construction of new impervious surfaces resulting in a net increase in impervious surfaces of approximately 12,630 SF. The project will require the implementation of erosion controls during construction to reduce the impacts of erosion and sedimentation and the installation of permanent stormwater management facilities to control the rate of discharge from the property.

The total disturbance for the project will be 0.9 acres. Therefore, coverage under the NYSDEC General SPDES Permit for Stormwater Discharges from Construction Projects. However, stormwater management facilities have been planned to meet the requirements of the Town of Wappinger and will provide some level of stormwater quality treatment.

#### 2.0 SITE DESCRIPTION

This section briefly describes existing and proposed hydrologic and hydraulic conditions at and around the project site as they relate to surface water management planning considerations. Subsequent sections contain a description of the manner in which site runoff will be managed to minimize effects on areas adjacent to the site.

#### **Location**

The parcel proposed to be disturbed for this development project comprises approximately 1.75 acres of land on the north side of Middlebush Road, across from Pleasant Lane. The improvements are proposed on Tax Parcels 6157-01-414840 and 396837. The land in the area surrounding the site consists of mixed uses of residential, institutional and commercial.

The watershed that contributes to the Off-Site Discharge Point (ODP) also includes a portion of the Wappingers Central Schools property and single family residential properties adjacent to the project property.

# **Topography**

The property generally slopes from south to north on the east side of the building and from north to south on the west side of the building. A drainage swale runs along the north side of the building from east to west and drains toward the off-site discharge point consisting of a culvert at the south west corner of the property that transmits flow into the County highway drainage system. Slopes are mostly less than 10% across the site. The adjacent Wappingers Central School property slopes west to east at less than 10% to the wooded area of the project property and adjacent eastern property.

#### **Land Cover**

The construction project is on a previously-developed property, consisting of land covers of the building, asphalt parking and wooded/grass areas. The off-site areas that contribute to the ODP are generally paved and grass areas associated with the Wappingers Central School parcel and additional wooded areas

#### Soils

According to maps from the National Cooperative Soil Survey for Dutchess County, the on-site soils within the project area are classified into the following mapping unit(s):

# Dutchess-Cardigan complex (DwB)

This soil is characterized as silt loam to a depth of 86 inches. Depth to groundwater and bedrock is greater than 80 inches. The hydrologic soil group is B/C and is characterized with moderate infiltration rates.

A 2-inch per hour infiltration rate was used to model the proposed storm features with infiltration capabilities. This rate will be confirmed with soil testing.

# Watercourses and Drainage Patterns

No streams are located on the property. The majority of the property drains to the south west corner discharge point. Off-site areas associated with the Wappingers Central School property generally sheet runoff into the wooded area of the project property and eastern property. This area drains along a ditch on the north side of the project building, and subsequently flows to the 12" RCP culvert at Middlebush. This drainage pattern will be continued with the proposed northern cut off swale/berm.

#### Regulated Wetlands

ACOE-regulated wetlands are present on the property and adjacent property to the east and are shown on the property survey.

# **Floodplains**

According to FEMA floodplain mapping, no floodplains are located on or adjacent to the property.

#### 3.0 METHODOLOGY / NYSDEC UNIFORM SIZING CRITERIA

The Environmental Protection Agency, New York State Department of Environmental Conservation and Town of Wappinger require the management of stormwater from construction projects to meet standards for water quantity. The project will result in the disturbance of less than one (1) acre and is therefore not subject to water quality treatment standards. However, water quality treatment will be provided for a portion of the runoff. Maintaining water quality involves the removal or reduction of pollutants including suspended solids, phosphates, nitrates and other chemicals generated by development. The water quantity standards require peak flow attenuation and include parameters designed to protect downstream channels, water bodies and properties from erosion and flooding.

#### Rainfall Data

Rainfall data utilized in the modeling and analysis was taken from the NYSDEC Stormwater Design Manual:

Storm Event	90% Rainfall Event*	1-yr	2-yr	5-yr	10-yr	25-yr	50-yr	100-yr
Precipitation <sub>24-hr</sub> P <sub>n-yr</sub> (inches)	1.4	2.7	-	-	4.9	-	-	9.0

**Table 1 - Precipitation Values** 

# Hydrologic and Hydraulic Analysis

The peak rate of stormwater runoff generated from the proposed improvements during the design storms was calculated to determine the required storage volume of the dry detention basin. The time of concentration (Tc)and runoff curve numbers (CN) were then calculated for each watershed area. A minimum Tc of 0.1 hour was selected due to the small area of the watershed. This data was then entered into the *HydroCAD* computer program for analysis. *HydroCAD*, a Computer-Aided-Design (CAD) program, was used to analyze the hydrologic and hydraulic characteristics of a given watershed and associated stormwater management facilities. It utilizes the latest techniques to predict the consequences of any given storm. *HydroCAD* has the capability of computing hydrographs (which represents discharge rates characteristic of specified watershed conditions, precipitation, and geologic factors) combining hydrographs and routing flows though pipes, streams and ponds. HydroCAD is used to calculate peak runoff flows and to create hydrographs for the various storm events evaluated for both pre-development and post development conditions.

#### Watershed Description

#### **Existing (Pre-Development) Watershed Conditions**

The study area consists of the portion of the property that will be altered as part of the improvements and the area contributing to the ODP culvert at Middlebush Road. The overall study area is 5.0 acres and the portion impacted by the project is 0.79 acres. All of the effected project area contributes runoff to a 12-inch RCP culvert that transmits flows from the south west corner of the property into the County Highway drainage system. The existing conditions includes an existing 6,700 sf +/- building and 6,100 sf asphalt parking area. The remaining areas are generally wooded/grass areas.

The Off-Site Discharge Point is the 12-inch RCP culvert that accepts runoff from the property and transmits it into the County system along Middlebush Road.

#### **Proposed (Post-Development) Watershed Conditions**

The post-development drainage area will be modified by the proposed improvements by converting wooded/grass areas to impervious surfaces related to the parking/truck maneuvering area expansion and sidewalk construction. The net increase in impervious area is 12,630 SF.

The subcatchments are described in the HydroCAD Figure. The subcatchments are generally described:

#### 1 Post: Area to the Infiltration Trench

This area consists of most of the parking area that will sheet flow runoff to an infiltration trench located to the north side of the parking area and building.

#### 2 Post: Area Direct to the Dry Detention Basin

This area consists of the existing building and proposed sidewalk along the west side of the building and area of the basin and immediately adjacent.

#### 3 Post: Remaining Area Direct to the ODP

This area consists of the off-site areas and the portion of the project property not impacted by the proposed construction.

#### 4 Post: Area to Drywells

This area consists of the asphalt area adjacent to the east side of the building.

The Off-Site Discharge Point is the 12-inch RCP culvert that accepts runoff from the property and transmits it into the County system along Middlebush Road.

# Proposed Water Quantity and Quality Controls

#### **Water Quantity**

The following table summarize the stormwater management system performance and discharge point parameters as found in the engineering calculations attached. The infiltration trench and drywells were modelled assuming an infiltration rate of 2 inches/hour.

	Pre-	Post-		
Design Point Summary	Development	Development	Units	Satisfied
Design Point 1				
Contributing Watershed Area	5.0	5.0	AC	
		Peak		
		Discharge		
1-year event	2.2	1.9	cfs	√
10-year event	7.8	7.3	cfs	√
100-year event	20.5	19.9	cfs	√

The off-site discharge point will not be significantly affected by the proposed project. A minor reduction in peak flow rates to the culvert will occur as a result of the proposed on-site detention. The attached figure indicates that the 12" RCP can pass the 1-yr storm event without significant ponding at the inlet. Larger flows will continue to pond on-site within the wetland until drained by the culvert.

#### Water Quality

The water quality volume is directly related to the amount of impervious surface created at a site. The water quality volume (**WQv**) is designed to improve water quality by treating 90% of the average annual stormwater runoff volume.

Although water quality treatment is not required, the storm system will provide some level of water quality treatment of runoff from 0.39 acres of the site's impervious surfaces. The stormwater management system is as follows:

#### 1. Infiltration Trench

The majority of the runoff from the parking area will sheet flow to the north to a 3'x3' stone infiltration trench. An overflow pipe will transmit larger flows to the dry detention basin.

#### 2. Dry Wells

Runoff from asphalt and gravel areas directly adjacent to the east side of the building will flow to three (3) ten ft. diameter drywells. The drywells will collect upto the 100-year runoff volume without surcharge. However, an overflow pipe has been provided to direct surcharges to the bioretention area.

#### 3. <u>Dry Detention Basin</u>

A dry detention basin is proposed on the west side of the building to provide quantity control. The dry detention basin will discharge to the wooded area adjacent to the off-site discharge point.

The proposed construction will not increase the approved developed peak discharge rates from the site and will provide some water quality treatment from the proposed project.

Sincerely,



Troy A. Wojciekofsky, P. E., LEED-AP Engineer Attachments: Soils Information HydroCAD Report

Web Soil Survey National Cooperative Soil Survey

Natural Resources Conservation Service

USDA

10/1/2019 Page 1 of 3

# MAP LEGEND

#### Special Line Features Streams and Canals Interstate Highways Aerial Photography Very Stony Spot Major Roads Local Roads US Routes Stony Spot Spoil Area Wet Spot Other Rails Water Features **Fransportation** Background W 8 ŧ Soil Map Unit Polygons Area of Interest (AOI) Miscellaneous Water Soil Map Unit Points Soil Map Unit Lines Closed Depression Marsh or swamp Perennial Water Mine or Quarry Special Point Features **Gravelly Spot Borrow Pit** Clay Spot Lava Flow **Gravel Pit** Area of Interest (AOI) Blowout Landfill Soils

# MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Dutchess County, New York Survey Area Data: Version 16, Sep 16, 2019

Survey Area Data: Version 16, Sep 16, 2019

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Oct 7, 2013—Feb 26,

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Severely Eroded Spot

Slide or Slip Sodic Spot

Sinkhole

Saline Spot Sandy Spot

Rock Outcrop

# **Map Unit Legend**

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
DwB	Dutchess-Cardigan complex, undulating, rocky	1.0	100.0%
Totals for Area of Interest		1.0	100.0%

# **Dutchess County, New York**

#### DwB—Dutchess-Cardigan complex, undulating, rocky

#### **Map Unit Setting**

National map unit symbol: 9rfn Elevation: 50 to 1,000 feet

Mean annual precipitation: 41 to 47 inches Mean annual air temperature: 45 to 50 degrees F

Frost-free period: 115 to 195 days

Farmland classification: All areas are prime farmland

#### **Map Unit Composition**

Dutchess and similar soils: 40 percent Cardigan and similar soils: 30 percent

Minor components: 30 percent

Estimates are based on observations, descriptions, and transects of

the mapunit.

#### **Description of Dutchess**

#### Setting

Landform: Hills, ridges

Landform position (two-dimensional): Summit Landform position (three-dimensional): Crest

Down-slope shape: Convex Across-slope shape: Convex

Parent material: Loamy till derived mainly from phyllite, slate,

schist, and shale

#### Typical profile

H1 - 0 to 8 inches: silt loam H2 - 8 to 28 inches: silt loam

H3 - 28 to 86 inches: channery silt loam

#### **Properties and qualities**

Slope: 1 to 6 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Well drained

Capacity of the most limiting layer to transmit water (Ksat):

Moderately high to high (0.57 to 1.98 in/hr) Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water storage in profile: High (about 9.6 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: B Hydric soil rating: No

#### **Description of Cardigan**

#### Setting

Landform: Hills, ridges

Landform position (two-dimensional): Summit Landform position (three-dimensional): Crest

Down-slope shape: Convex Across-slope shape: Convex

Parent material: Loamy till or colluvium derived from phyllite, slate,

shale, and schist

#### **Typical profile**

H1 - 0 to 8 inches: channery silt loam
H2 - 8 to 20 inches: channery loam
H3 - 20 to 30 inches: channery silt loam
H4 - 30 to 34 inches: unweathered bedrock

#### **Properties and qualities**

Slope: 1 to 6 percent

Depth to restrictive feature: 20 to 40 inches to lithic bedrock

Natural drainage class: Well drained

Capacity of the most limiting layer to transmit water (Ksat): Low to

moderately low (0.00 to 0.06 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water storage in profile: Low (about 4.1 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: C Hydric soil rating: No

#### **Minor Components**

#### Georgia

Percent of map unit: 10 percent

Hydric soil rating: No

#### Massena

Percent of map unit: 9 percent

Hydric soil rating: No

#### Nassau

Percent of map unit: 9 percent

Hydric soil rating: No

#### Sun

Percent of map unit: 1 percent Landform: Depressions Hydric soil rating: Yes

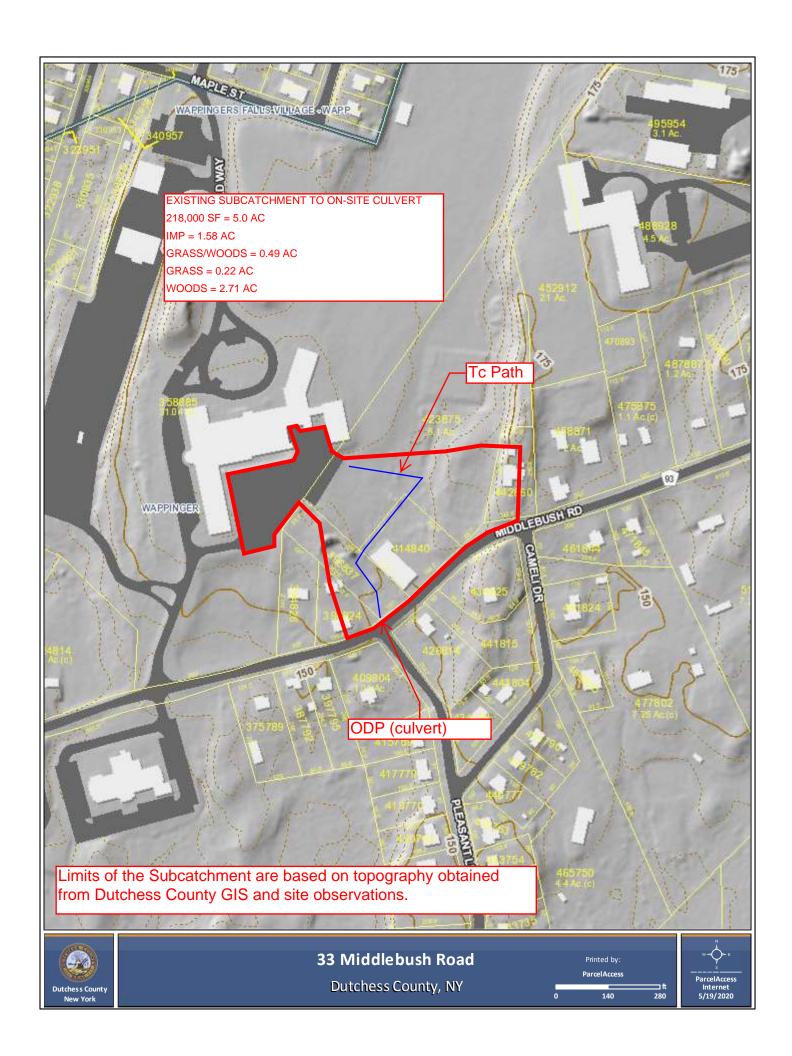
#### **Rock outcrop**

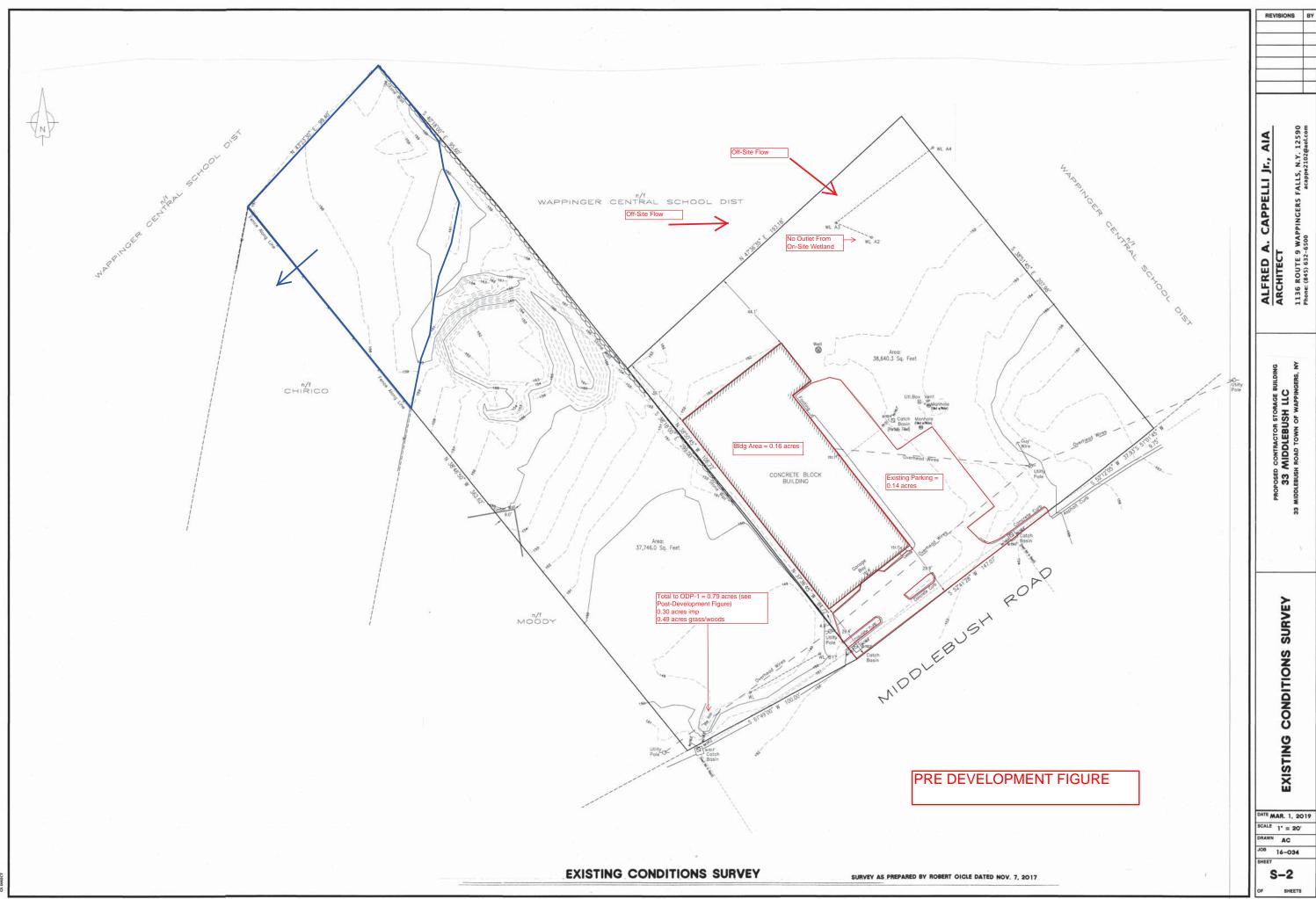
Percent of map unit: 1 percent

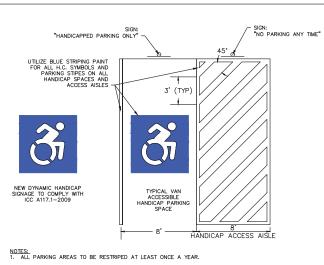
Hydric soil rating: Unranked

# **Data Source Information**

Soil Survey Area: Dutchess County, New York Survey Area Data: Version 16, Sep 16, 2019

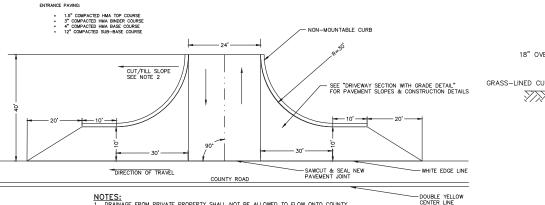






- 2. ALL STRIPING TO BE 4" WIDE UNLESS NOTED OTHERWISE.
- 3. SEE SIGN DETAILS FOR SIGN MOUNTING AND SIGN HEIGHT REQUIREMENTS.

TYPICAL HANDICAP PARKING STRIPING DETAIL



YARD INLET

YARD INLET

INV. = 149.0

SIN = 0.25 AC

OUTLET STRUCTURE - 1 R = 150.5

25 LF 12" PERFORATED
SICPP EQUALIZER PIPE ® 0%
INV = 149.5 -

PROPOSED ONE STORY BUILDING 6,890 S.F. 152.5 — FF = 152.50

CB-3 R=152.2 10' DIA. 152.5 — SEEPAGE PIT —

37 LF 10" SDR 35 PVC @ 0.9% -

INV OUT = 148.8

NOTES:

1. DRAINAGE FROM PRIVATE PROPERTY SHALL NOT BE ALLOWED TO FLOW ONTO COUNTY HIGHWAY. IF NECESSARY, PERMITEE WILL BE REQUIRED TO INSTALL DRAINAGE SYSTEM AT PROPOSED DRIVEWAY(S).

- CUT/FILL SLOPES SHALL BE CONSTRUCTED NO STEEPER THAN 2 HORIZONTAL TO 1 VERTICAL (2:1). DCDPW SHALL EVALUATE CUT/FILL SLOPES FOR ACCEPTABILITY.
- 3. DIMENSIONS SHOWN ARE TYPICAL. FIELD CONDITIONS MAY REQUIRE ADJUSTMENTS TO MATCH EXISTING CONDITIONS PER DCDPW DIRECTION.

# DCDPW LOCAL ROAD/SUBDIVISION ROAD DETAIL NOT TO SCALE

ES-3-INV. =-148.8

DETENTION BASIN

148.8

END SECTION WITH RIP
RAP PROTECTION (TYP)

149.8 <sup>\_/</sup>

16 LF 12 SICPP @ 0.6% -

INV. = 148.7

148.5 -

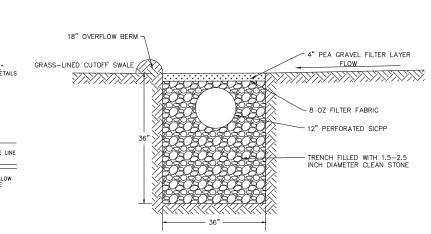
ES-1 INV. = 148.5

LIMITS OF DISTURBANCE: 39,650 SF

1,420 SF WETLAND DISTURBANCE -

2' WIDE X 6" DEEP GRASS-LINED \$WALE

ISTING 12" RCP



PERF. PIPE IN INFILTRATION TRENCH

- 12" PERF. PE PIPE, 150 LF CONNECT TO YARD INLET IN 3' WIDE X 3' DEEP INFILTRATION TRENCH

TOP ELEV. = 151.0 BOTTOM ELEV. = 148

3,190 SF WETLAND DISTURBANCE

OST TO INFILTRATION

RENCH = 0.17 AC MP = 0.04 AC

8" SI GRAVEL = ROOF LEADER HEADER @ 1.0%

CB41 R=152<del>.2</del>

10' DIA. SEEPAGE PI

SEEPAGE P

BC = 152.9 -

94

98

Asphalt Curb

S 52'12'05" W 37.9S 51'01'45" W

9.75

152.4

TC = 153.4 BC = 152.9

5' WIDE FLUSH CURB

GRADING AND DRAINAGE PLAN

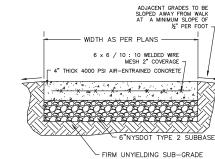
S 52°4 28" W 147.07'

MIDDLEBUSH ROAD

RYWELLS = 0.22 AC

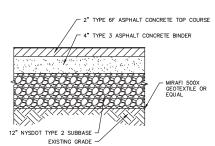
CUTOFF SWALE TO INTERCEPT RUNOFF FROM WCSD PROPERTY

PROPOSED ASPHALT PARKING

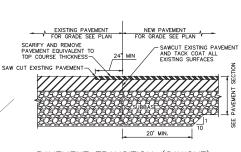


- EXPANSION JOINTS OF  $\frac{1}{2}$ " CELLULOSE OR SIMILAR WITH  $\frac{1}{2}$ " RADIUS EDGES SHALL BE PLACED ACROSS THE FULL WIDTH OF WALKWAY, AT INTERVALS NOT TO EXCEED 20' ALONG LENGTH OF SIDEWALK.

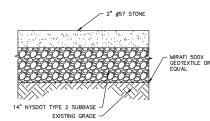
#### SIDEWALK DETAIL



ASPHALT PARKING PAVEMENT DETAIL



PAVEMENT TRANSITION (SAWCUT)



GRAVEL PARKING PAVEMENT DETAIL

POST-DEVELOPMENT FIGURE

P.O. BOX 913 WAPPINGER FALLS, NY 12590 845-594-1529

TW ENGINEERING, P.C.

BUILDING YORK NEW 33 MIDDLEBUSH ROAD VN OF WAPPINGER, NEV STORAG 33 MIDDLEBUSH LLC ONTRACTOR

DATE: PROJECT NO 2022-20 SCALE:

GRADING & DRAINAGE P 1' = 20'

S-4.0

5/31/2020

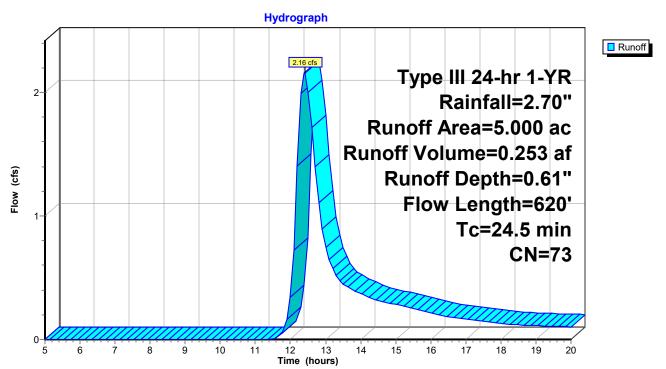
# **Subcatchment 1 Pre: Total to ODP-1**

Runoff = 2.16 cfs @ 12.40 hrs, Volume= 0.253 af, Depth= 0.61"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.10 hrs Type III 24-hr 1-YR Rainfall=2.70"

Area	(ac) C	N Desc	cription							
1.	.580	98 Pave	Paved parking & roofs							
0.	.490 (	35 Woo	Voods/grass comb., Fair, HSG B							
0	.220	59 50-7	5% Grass	cover, Fair	r, HSG B					
2	.710	60 Woo	ds, Fair, F	ISG B						
5.	.000	73 Wei	ghted Aver	age						
Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
7.2	100	0.0400	0.2		Sheet Flow,					
					Grass: Short n= 0.150 P2= 3.40"					
17.3	520	0.0100	0.5		Shallow Concentrated Flow,					
					Woodland Kv= 5.0 fps					
24.5	620	Total								

# **Subcatchment 1 Pre: Total to ODP-1**



5/31/2020

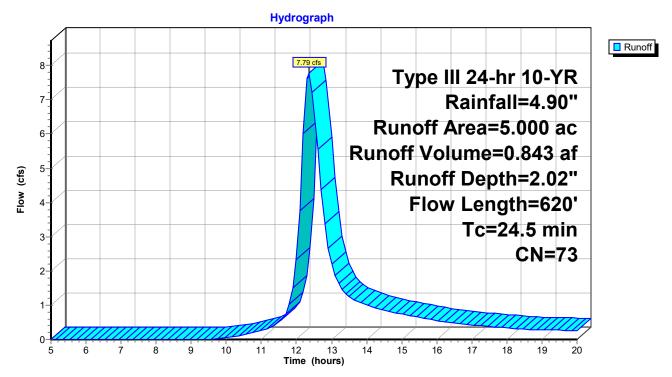
#### Subcatchment 1 Pre: Total to ODP-1

Runoff = 7.79 cfs @ 12.36 hrs, Volume= 0.843 af, Depth= 2.02"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.10 hrs Type III 24-hr 10-YR Rainfall=4.90"

Area	(ac) C	N Desc	cription							
1.	.580	98 Pave	Paved parking & roofs							
0.	.490 (	35 Woo	Voods/grass comb., Fair, HSG B							
0	.220	59 50-7	5% Grass	cover, Fair	r, HSG B					
2	.710	60 Woo	ds, Fair, F	ISG B						
5.	.000	73 Wei	ghted Aver	age						
Tc	Length	Slope	Velocity	Capacity	Description					
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
7.2	100	0.0400	0.2		Sheet Flow,					
					Grass: Short n= 0.150 P2= 3.40"					
17.3	520	0.0100	0.5		Shallow Concentrated Flow,					
					Woodland Kv= 5.0 fps					
24.5	620	Total								

# **Subcatchment 1 Pre: Total to ODP-1**



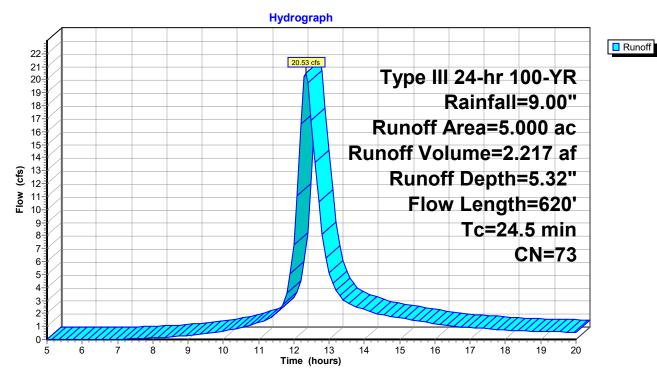
#### **Subcatchment 1 Pre: Total to ODP-1**

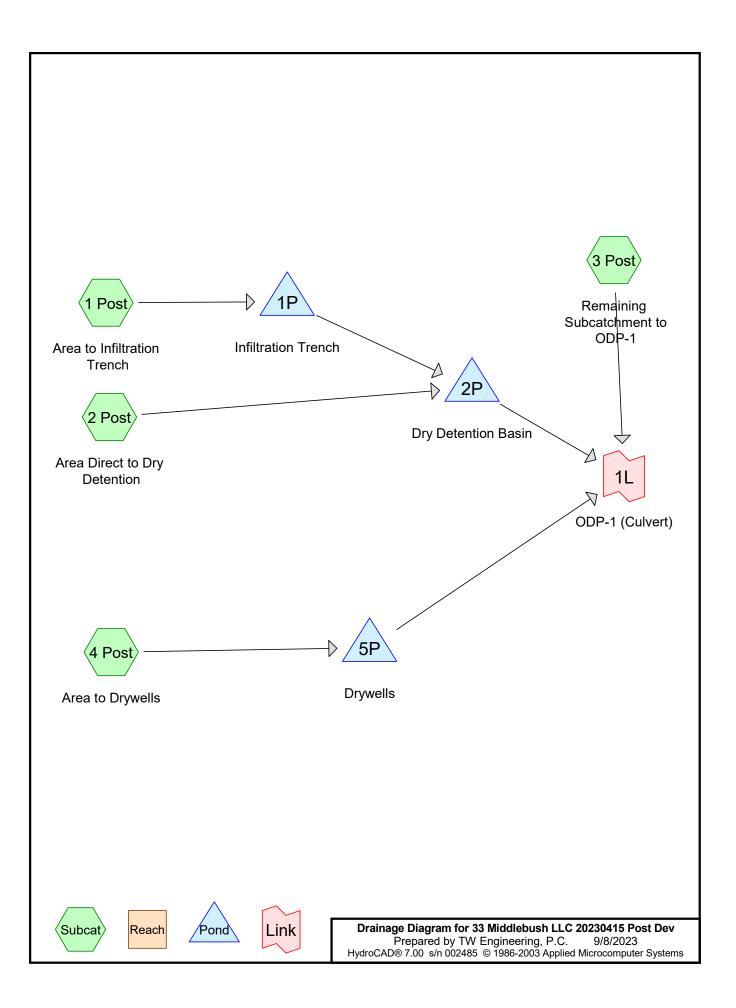
Runoff = 20.53 cfs @ 12.34 hrs, Volume= 2.217 af, Depth= 5.32"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.10 hrs Type III 24-hr 100-YR Rainfall=9.00"

Area	(ac)	CN	Desc	cription						
1	.580	98	Pave	Paved parking & roofs						
0	.490	65	Woo	ds/grass c	omb., Fair,	HSG B				
0	.220	69	50-7	5% Grass	cover, Fair	, HSG B				
2	.710	60	Woo	ds, Fair, H	ISG B					
5	.000	73	Weig	ghted Aver	age					
Tc	Lengt	h :	Slope	Velocity	Capacity	Description				
(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)					
7.2	10	0 0	.0400	0.2		Sheet Flow,				
						Grass: Short n= 0.150 P2= 3.40"				
17.3	52	0 0	.0100	0.5		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
24.5	62	0 T	otal				,			

# **Subcatchment 1 Pre: Total to ODP-1**





#### 33 Middlebush LLC 20230415 Post Dev

Type III 24-hr 1-YR Rainfall=2.70"

Prepared by TW Engineering, P.C.

Page 2

HydroCAD® 7.00 s/n 002485 © 1986-2003 Applied Microcomputer Systems

9/8/2023

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1 Post: Area to Infiltration Trench

Runoff Area=0.170 ac Runoff Depth=1.24"

Tc=6.0 min CN=85 Runoff=0.26 cfs 0.018 af

**Subcatchment 2 Post: Area Direct to Dry Detention** 

Runoff Area=0.250 ac Runoff Depth=1.53" Tc=6.0 min CN=89 Runoff=0.47 cfs 0.032 af

**Subcatchment 3 Post: Remaining Subcatchment to ODP-1** 

Runoff Area=4.360 ac Runoff Depth=0.57"

Flow Length=620' Tc=24.5 min CN=72 Runoff=1.74 cfs 0.206 af

**Subcatchment 4 Post: Area to Drywells** 

Runoff Area=0.220 ac Runoff Depth=1.45"

Tc=6.0 min CN=88 Runoff=0.39 cfs 0.027 af

**Pond 1P: Infiltration Trench** 

Peak Elev=149.14' Storage=258 cf Inflow=0.26 cfs 0.018 af

Discarded=0.03 cfs 0.016 af Primary=0.06 cfs 0.002 af Outflow=0.08 cfs 0.018 af

Pond 2P: Dry Detention Basin

Peak Elev=149.16' Storage=441 cf Inflow=0.47 cfs 0.034 af

Outflow=0.18 cfs 0.032 af

Pond 5P: Drywells

Peak Elev=147.29' Storage=0.014 af Inflow=0.39 cfs 0.027 af

Discarded=0.02 cfs 0.017 af Primary=0.00 cfs 0.000 af Outflow=0.02 cfs 0.017 af

Link 1L: ODP-1 (Culvert)

Inflow=1.92 cfs 0.238 af

Primary=1.92 cfs 0.238 af

Total Runoff Area = 5.000 ac Runoff Volume = 0.282 af Average Runoff Depth = 0.68"

9/8/2023

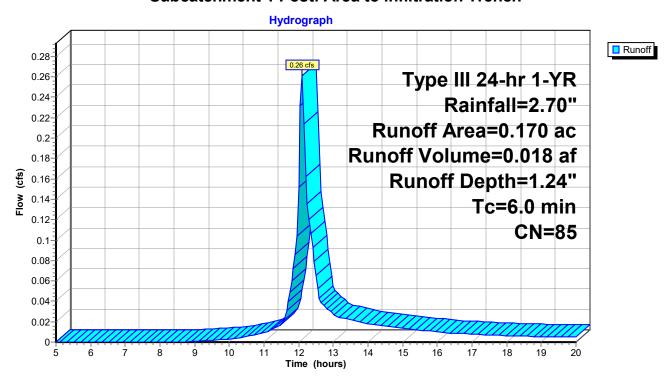
# **Subcatchment 1 Post: Area to Infiltration Trench**

Runoff = 0.26 cfs @ 12.09 hrs, Volume= 0.018 af, Depth= 1.24"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 1-YR Rainfall=2.70"

Area	(ac)	CN	Desc	cription		
0	.040	98	Pave	ed parking	& roofs	
0.	.030	65	Woo	ds/grass c	omb., Fair,	, HSG B
0	.100	85	Grav	el roads, l	HSG B	
0.	.170	85	Weig	ghted Aver	age	
Tc	Leng	th	Slope	Velocity	Capacity	Description
(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)	
6.0						Direct Entry,

# **Subcatchment 1 Post: Area to Infiltration Trench**



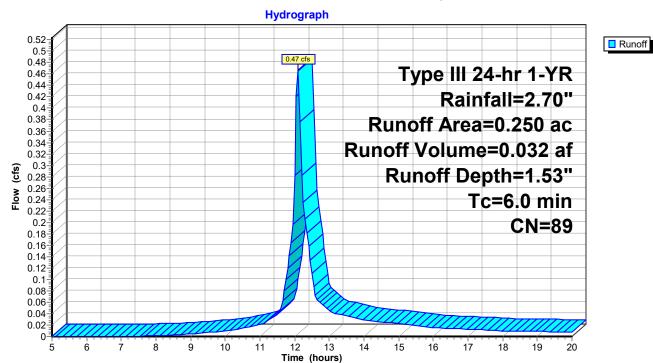
# **Subcatchment 2 Post: Area Direct to Dry Detention**

Runoff = 0.47 cfs @ 12.09 hrs, Volume= 0.032 af, Depth= 1.53"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 1-YR Rainfall=2.70"

Area	(ac)	CN	Desc	ription		
0.	170	98				
0.	.080	69	50-7	5% Grass	cover, Fair	r, HSG B
0.	250	89	Weig	ghted Aver	age	
Tc (min)	Lengt		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	(100	·- <i>)</i>	(10/10)	(1000)	(010)	Direct Entry,

# **Subcatchment 2 Post: Area Direct to Dry Detention**



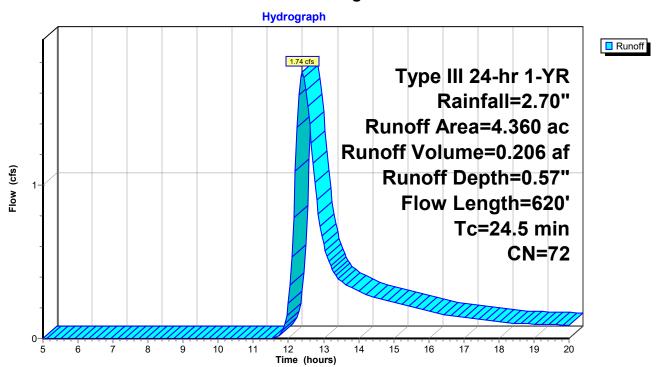
# **Subcatchment 3 Post: Remaining Subcatchment to ODP-1**

Runoff = 1.74 cfs @ 12.40 hrs, Volume= 0.206 af, Depth= 0.57"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 1-YR Rainfall=2.70"

	Area (	(ac) C	N Desc	cription							
	1.	320 9	98 Pave	Paved roads w/curbs & sewers							
	0.	220 6	§9 50-7	50-75% Grass cover, Fair, HSG B							
	2.	750 6	30 Woo	ds, Fair, F	ISG B						
	0.	070 6	31 >75°	% Grass co	over, Good	, HSG B					
	4.	360	72 Weig	ghted Aver	age						
					_						
	Tc	Length	Slope	Velocity	Capacity	Description					
<u>(r</u>	min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	7.2	100	0.0400	0.2		Sheet Flow,					
						Grass: Short n= 0.150 P2= 3.40"					
1	17.3	520	0.0100	0.5		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
2	24.5	620	Total								

# **Subcatchment 3 Post: Remaining Subcatchment to ODP-1**



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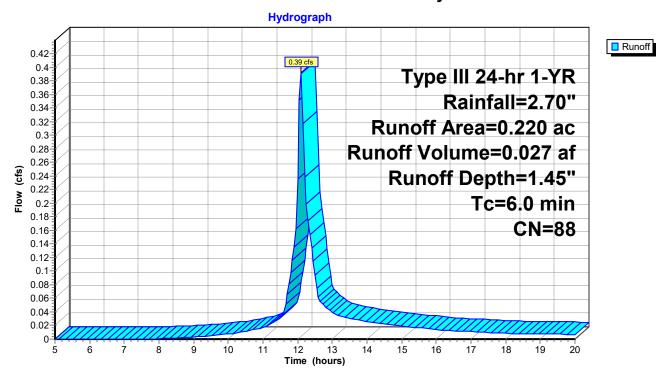
# **Subcatchment 4 Post: Area to Drywells**

Runoff = 0.39 cfs @ 12.09 hrs, Volume= 0.027 af, Depth= 1.45"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 1-YR Rainfall=2.70"

Are	ea (ac)	CN	Desc	cription		
	0.120	98				
	0.060	85	Grav	el roads, l	HSG B	
	0.040	61	>759	% Grass co	over, Good	I, HSG B
	0.220	88	Wei	ghted Aver	age	
٦	c Len	gth	Slope	Velocity	Capacity	Description
(mi	า) (fe	eet)	(ft/ft)	(ft/sec)	(cfs)	
6	.0					Direct Entry,

# **Subcatchment 4 Post: Area to Drywells**



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#### **Pond 1P: Infiltration Trench**

Inflow Area =	0.170 ac, Inflow Depth = 1.24"	for 1-YR event
Inflow =	0.26 cfs @ 12.09 hrs, Volume=	0.018 af
Outflow =	0.08 cfs @ 12.45 hrs, Volume=	0.018 af, Atten= 69%, Lag= 21.2 min
Discarded =	0.03 cfs @ 11.70 hrs, Volume=	0.016 af
Primary =	0.06 cfs @ 12.45 hrs, Volume=	0.002 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 149.14' @ 12.45 hrs Surf.Area= 564 sf Storage= 258 cf Plug-Flow detention time= 74.6 min calculated for 0.018 af (100% of inflow) Center-of-Mass det. time= 73.7 min (870.4 - 796.8)

#	Invert	Avail.S	torage Stora	na D	escription					
				Storage Description						
1	148.00'			<b>U</b> ,						
			1,692	1,692 cf Overall x 40.0% Voids						
Elev	/ation	Surf.Area	Inc.S	tore	Cum.Store					
	(feet)	(sq-ft)	(cubic-f	eet)	(cubic-feet)					
1	148.00 564			0	0					
	151.00 564			,692	1,692					
•	01.00	00.	• :	,00_	.,002					
#	Routing	Invert	Outlet Device	es						
1	Discarded	0.00'	0.002780 fpr	n Exfi	Itration over entire S	urface area				
2	Primary	151.00'	•		readth Broad-Creste					
	,		•		0.40 0.60 0.80 1.00	•				
			` ,							
3	Drimary	149.00'	Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 <b>12.0" x 52.0' long Culvert</b> CPP, projecting, no headwall, Ke= 0.900							
3	Primary	143.00		_		•				
			Outlet invert	= 148	.80' S= 0.0038 '/' n=	= 0.012				

**Discarded OutFlow** Max=0.03 cfs @ 11.70 hrs HW=148.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.03 cfs)

**Primary OutFlow** Max=0.06 cfs @ 12.45 hrs HW=149.14' (Free Discharge)

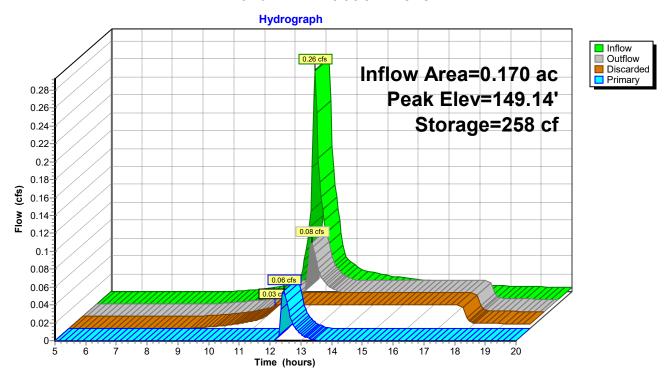
2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-3=Culvert (Barrel Controls 0.06 cfs @ 1.2 fps)

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# **Pond 1P: Infiltration Trench**



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# Pond 2P: Dry Detention Basin

Inflow Area = 0.420 ac, Inflow Depth = 0.96" for 1-YR event Inflow = 0.47 cfs @ 12.09 hrs, Volume= 0.034 af

Outflow = 0.18 cfs @ 12.46 hrs, Volume= 0.032 af, Atten= 61%, Lag= 22.0 min

Primary = 0.18 cfs @ 12.46 hrs, Volume= 0.032 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 149.16' @ 12.46 hrs Surf.Area= 1,280 sf Storage= 441 cf Plug-Flow detention time= 53.4 min calculated for 0.032 af (96% of inflow)

Center-of-Mass det. time= 37.6 min ( 820.0 - 782.4 )

#	Invert	Avail.Storage		Storage Des	scription			
1	148.80'	3,	635 cf	<b>Custom Sta</b>	ige Data (Prisma	atic) Listed below		
Flov	ation	Surf.Area		Inc.Store	Cum.Store			
	(feet)	(sq-ft)		cubic-feet)	(cubic-feet)			
	48.80	900		0	0			
	49.00	1,200		210	210			
15	50.00	1,700		1,450	1,660			
15	51.00	2,250		1,975	3,635			
#	Routing	Invert	Outlet	Devices				
1	Primary	150.10'	12.0' lc	ng x 8.0' bre	eadth Broad-Cro	ested Rectangular Weir		
	-		Head (	feet) 0.20 0.	.40 0.60 0.80 1	1.00 1.20 1.40 1.60 1.80 2.00 2.50		
					50 5.00 5.50			
			Coef. (	English) 2.43	3 2.54 2.70 2.6	9 2.68 2.68 2.66 2.64 2.64 2.64 2.65		
			2.65 2.66 2.68 2.70 2.74					
2	Primary	148.80'	4.0" Vert. Orifice/Grate C= 0.600					
3	Primary	149.50'		•		rested Rectangular Weir		
						1.00 1.20 1.40 1.60		
			Coef. (	English) 2.49	9 2.56 2.70 2.6	9 2.68 2.69 2.67 2.64		

**Primary OutFlow** Max=0.18 cfs @ 12.46 hrs HW=149.16' (Free Discharge)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

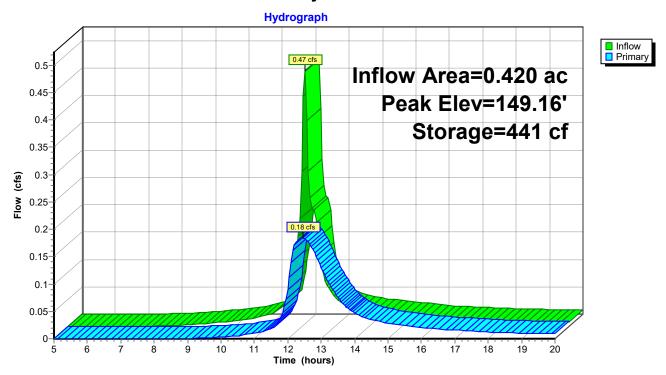
**—2=Orifice/Grate** (Orifice Controls 0.18 cfs @ 2.1 fps)

-3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

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# Pond 2P: Dry Detention Basin



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# Pond 5P: Drywells

Inflow Area =	0.220 ac, Inflow Depth = 1.45"	for 1-YR event
Inflow =	0.39 cfs @ 12.09 hrs, Volume=	0.027 af
Outflow =	0.02 cfs @ 11.40 hrs, Volume=	0.017 af, Atten= 95%, Lag= 0.0 min
Discarded =	0.02 cfs @ 11.40 hrs, Volume=	0.017 af
Primary =	0.00 cfs @ 5.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 147.29' @ 14.63 hrs Surf.Area= 0.011 ac Storage= 0.014 af Plug-Flow detention time= 191.0 min calculated for 0.017 af (64% of inflow) Center-of-Mass det. time= 119.5 min (906.9 - 787.4)

#	Invert	Avail.Storage		Storage Description						
1	146.00'	0.022 af		10.00'D x 4.00'H Vertical Cone/Cylinder x 3 Inside #2						
2	145.00'	0.013 af		14.00'D x 5.00'H Vertical Cone/Cylinder x 3						
				0.053 af Overall - 0.022 af Embedded = 0.031 af x 40.0% Voids						
	0.034 af Total Available Storage									
#	Routing	Invert	Outle	t Devices						
1	Discarded	0.00'	0.002	0.002780 fpm Exfiltration over entire Surface area						
2	Primary	150.50'	12.0"	<b>12.0" x 115.0' long Culvert</b> RCP, sq.cut end projecting, Ke= 0.500						
	•		Outle	Outlet Invert= 149.00' S= 0.0130 '/' n= 0.012 Cc= 0.900						

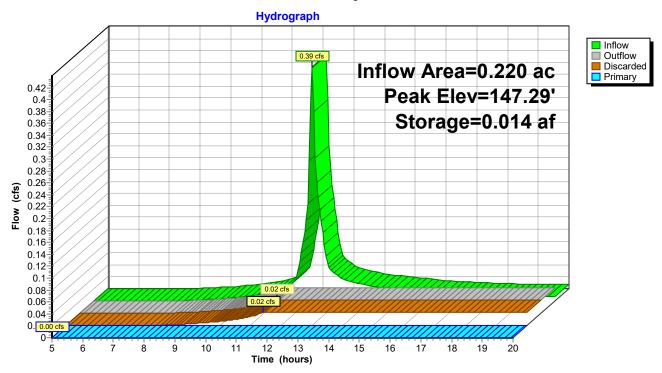
**Discarded OutFlow** Max=0.02 cfs @ 11.40 hrs HW=145.07' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=145.00' (Free Discharge) 2=Culvert (Controls 0.00 cfs)

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# Pond 5P: Drywells



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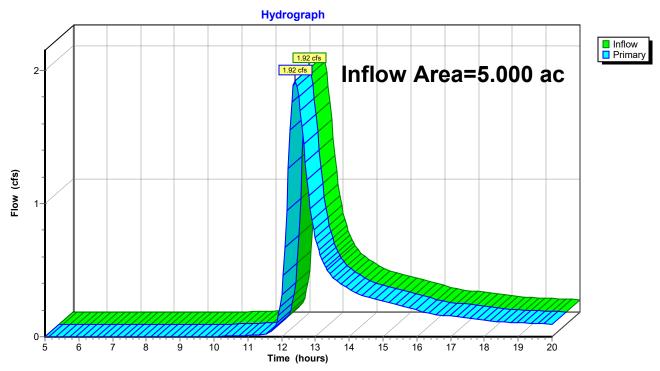
# Link 1L: ODP-1 (Culvert)

Inflow Area = 5.000 ac, Inflow Depth = 0.57" for 1-YR event 1.92 cfs @ 12.40 hrs, Volume= 0.238 af

Primary = 1.92 cfs @ 12.40 hrs, Volume= 0.238 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

# Link 1L: ODP-1 (Culvert)



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Type III 24-hr 10-YR Rainfall=4.90"

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1 Post: Area to Infiltration Trench Runoff Area=0.170 ac Runoff Depth=3.08"

Tc=6.0 min CN=85 Runoff=0.63 cfs 0.044 af

Subcatchment 2 Post: Area Direct to Dry Detention Runoff Area=0.250 ac Runoff Depth=3.47"

Tc=6.0 min CN=89 Runoff=1.02 cfs 0.072 af

**Subcatchment 3 Post: Remaining Subcatchment to ODP-1**Runoff Area=4.360 ac Runoff Depth=1.94"

Flow Length=620' Tc=24.5 min CN=72 Runoff=6.60 cfs 0.706 af

Subcatchment 4 Post: Area to Drywells Runoff Area=0.220 ac Runoff Depth=3.37"

Tc=6.0 min CN=88 Runoff=0.88 cfs 0.062 af

Pond 1P: Infiltration Trench Peak Elev=149.47' Storage=332 cf Inflow=0.63 cfs 0.044 af

Discarded=0.03 cfs 0.023 af Primary=0.57 cfs 0.019 af Outflow=0.60 cfs 0.043 af

Pond 2P: Dry Detention Basin Peak Elev=149.60' Storage=1,077 cf Inflow=1.57 cfs 0.092 af

Outflow=1.10 cfs 0.090 af

Pond 5P: Drywells Peak Elev=150.67' Storage=0.034 af Inflow=0.88 cfs 0.062 af

Discarded=0.02 cfs 0.021 af Primary=0.13 cfs 0.008 af Outflow=0.15 cfs 0.029 af

Link 1L: ODP-1 (Culvert) Inflow=7.30 cfs 0.804 af

Primary=7.30 cfs 0.804 af

Total Runoff Area = 5.000 ac Runoff Volume = 0.884 af Average Runoff Depth = 2.12"

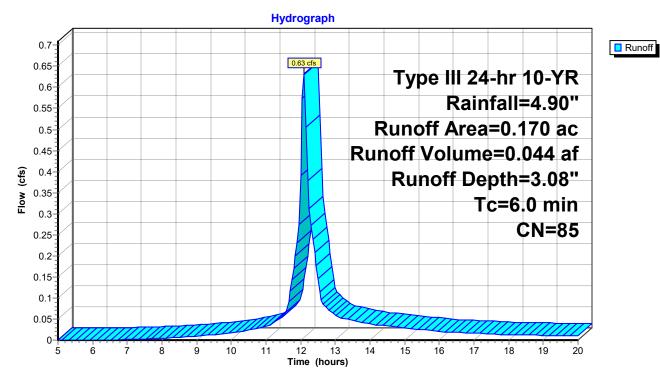
# **Subcatchment 1 Post: Area to Infiltration Trench**

Runoff = 0.63 cfs @ 12.09 hrs, Volume= 0.044 af, Depth= 3.08"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-YR Rainfall=4.90"

	Area	(ac)	CN	Desc	ription					
	0.	040								
	0.	030	65	Woo	Woods/grass comb., Fair, HSG B					
	0.100 85			Grav	el roads, l	HSG B				
0.170 85 Weighted Average										
	Tc	Leng		Slope	Velocity	Capacity	·			
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)		_		
	6.0						Direct Entry.			

# **Subcatchment 1 Post: Area to Infiltration Trench**



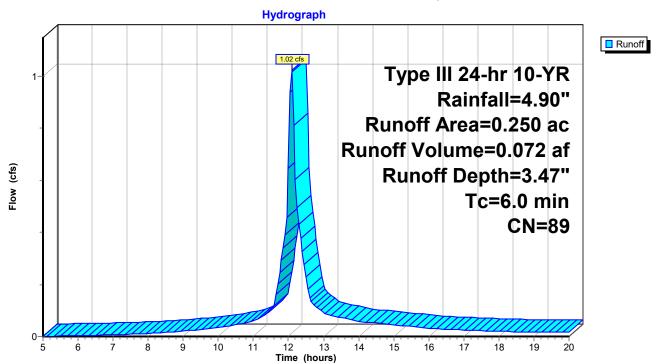
# **Subcatchment 2 Post: Area Direct to Dry Detention**

Runoff = 1.02 cfs @ 12.09 hrs, Volume= 0.072 af, Depth= 3.47"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-YR Rainfall=4.90"

_	Area	(ac)	CN	Desc	cription		
	0.	170	98				
	0.	080	69	50-7	5% Grass	cover, Fair	r, HSG B
	0.250 89 Weighted Average						
	т.	1	.41-	01	\/-l:\h.	Oih.	Description
		Leng	Įτη	Slope	Velocity	Capacity	Description
	(min)	(fe	et)	(ft/ft)	(ft/sec)	(cfs)	
	6.0						Direct Entry,

# **Subcatchment 2 Post: Area Direct to Dry Detention**



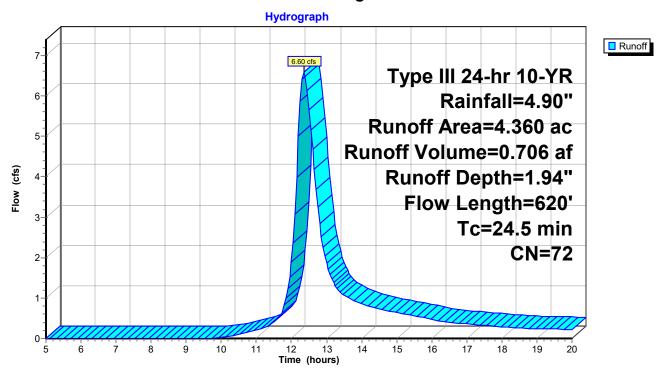
# **Subcatchment 3 Post: Remaining Subcatchment to ODP-1**

Runoff = 6.60 cfs @ 12.35 hrs, Volume= 0.706 af, Depth= 1.94"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-YR Rainfall=4.90"

Area	(ac) C	N Des	cription			
1.	320	98 Pave	ed roads w	/curbs & se	ewers	
0.	220	39 50 <b>-</b> 7	5% Grass	cover, Fair	r, HSG B	
2.	750	30 Woo	ods, Fair, F	ISG B		
0.	.070	31 >75°	% Grass co	over, Good	, HSG B	
4.	360	72 Wei	ghted Aver	age		
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
7.2	100	0.0400	0.2		Sheet Flow,	
					Grass: Short n= 0.150 P2= 3.40"	
17.3	520	0.0100	0.5		Shallow Concentrated Flow,	
					Woodland Kv= 5.0 fps	
24.5	620	Total				

# **Subcatchment 3 Post: Remaining Subcatchment to ODP-1**



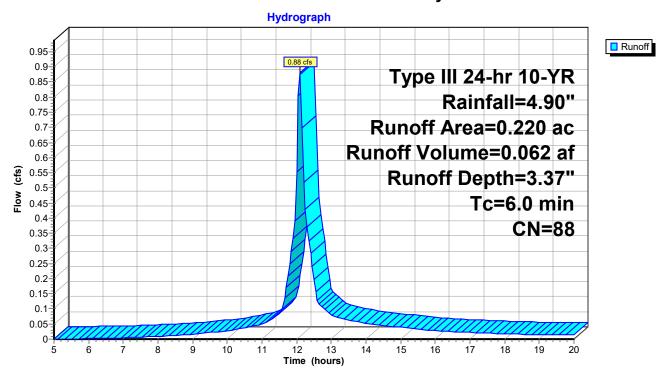
# **Subcatchment 4 Post: Area to Drywells**

Runoff = 0.88 cfs @ 12.09 hrs, Volume= 0.062 af, Depth= 3.37"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10-YR Rainfall=4.90"

	Area	(ac)	CN	Desc	cription					
	0.120 98									
	0.060		85	Grav	Gravel roads, HSG B					
_	0.040		61	>75%	% Grass co	over, Good	d, HSG B			
	0.	220	88	Weig	ghted Aver	age				
	Тс	Leng		Slope	Velocity	Capacity	·			
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)				
	6.0						Direct Entry.			

# **Subcatchment 4 Post: Area to Drywells**



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#### **Pond 1P: Infiltration Trench**

Inflow Area =	0.170 ac, Inflow Depth = 3.08"	for 10-YR event
Inflow =	0.63 cfs @ 12.09 hrs, Volume=	0.044 af
Outflow =	0.60 cfs @ 12.12 hrs, Volume=	0.043 af, Atten= 5%, Lag= 1.9 min
Discarded =	0.03 cfs @ 10.75 hrs, Volume=	0.023 af
Primary =	0.57 cfs @ 12.12 hrs, Volume=	0.019 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 149.47' @ 12.12 hrs Surf.Area= 564 sf Storage= 332 cf Plug-Flow detention time= 53.1 min calculated for 0.043 af (98% of inflow) Center-of-Mass det. time= 43.6 min (819.4 - 775.8)

#	Invert	Avail.S	torage Stora	na D	escription			
				Storage Description				
1	148.00'				tage Data (Prismatic)	Listed below		
			1,692	2 ct O	verall x 40.0% Voids			
Elev	/ation	Surf.Area	Inc.S	tore	Cum.Store			
	(feet)	(sq-ft)	(cubic-f	eet)	(cubic-feet)			
1	48.00	564		0	0			
	51.00	564		,692	1,692			
•	01.00	00.	• :	,00_	.,002			
#	Routing	Invert	Outlet Device	es				
1	Discarded	0.00'	0.002780 fpr	n Exfi	Itration over entire S	urface area		
2	Primary	151.00'	•		readth Broad-Creste			
	,		•	Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60				
			` ,					
3	Drimary	149.00'	, ,	Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 <b>12.0" x 52.0' long Culvert</b> CPP, projecting, no headwall, Ke= 0.900				
3	Primary	143.00		_		•		
			Outlet invert	= 148	.80' S= 0.0038 '/' n=	= 0.012		

**Discarded OutFlow** Max=0.03 cfs @ 10.75 hrs HW=148.03' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.03 cfs)

Primary OutFlow Max=0.55 cfs @ 12.12 hrs HW=149.46' (Free Discharge)

—2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

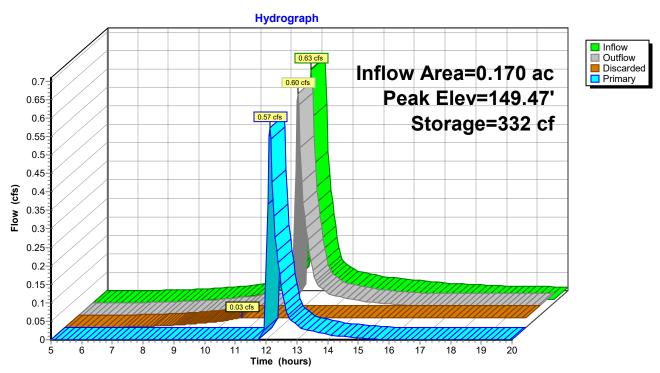
-3=Culvert (Barrel Controls 0.55 cfs @ 2.3 fps)

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### **Pond 1P: Infiltration Trench**



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#### Pond 2P: Dry Detention Basin

Inflow Area = 0.420 ac, Inflow Depth = 2.62" for 10-YR event Inflow = 1.57 cfs @ 12.10 hrs, Volume= 0.092 af

Outflow = 1.10 cfs @ 12.21 hrs, Volume= 0.090 af, Atten= 30%, Lag= 6.7 min

Primary = 1.10 cfs @ 12.21 hrs, Volume= 0.090 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 149.60' @ 12.21 hrs Surf.Area= 1,499 sf Storage= 1,077 cf Plug-Flow detention time= 42.4 min calculated for 0.089 af (97% of inflow)

Center-of-Mass det. time= 33.8 min ( 795.4 - 761.6 )

#	Invert	Avail.S	torage Storag	e Description	
1	148.80'	3,	635 cf Custor	m Stage Data (Prismatic) Listed below	
	ation (feet)	Surf.Area (sq-ft)			
14	18.80	900	•	0 0	
14	19.00	1,200	2	10 210	
15	50.00	1,700	1,4	50 1,660	
15	51.00	2,250	1,9	75 3,635	
#	Routing	Invert	Outlet Devices	<b>.</b>	
1	Primary	150.10'		.0' breadth Broad-Crested Rectangular Weir	
			` ,	20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50	
				0 4.50 5.00 5.50	
				) 2.43 2.54 2.70 2.69 2.68 2.68 2.66 2.64 2.64 2.64 2.65	
				6 2.68 2.70 2.74	
2	Primary	148.80'		ice/Grate C= 0.600	
3	Primary	149.50'			
				20 0.40 0.60 0.80 1.00 1.20 1.40 1.60	
			Coef. (English)	) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64	

**Primary OutFlow** Max=1.04 cfs @ 12.21 hrs HW=149.59' (Free Discharge)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

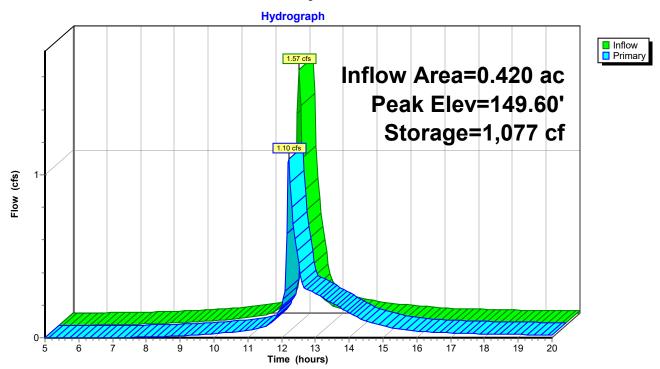
**—2=Orifice/Grate** (Orifice Controls 0.33 cfs @ 3.8 fps)

—3=Broad-Crested Rectangular Weir (Weir Controls 0.70 cfs @ 0.8 fps)

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# Pond 2P: Dry Detention Basin



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## Pond 5P: Drywells

Inflow Area =	0.220 ac, Inflow Depth = 3.37"	for 10-YR event
Inflow =	0.88 cfs @ 12.09 hrs, Volume=	0.062 af
Outflow =	0.15 cfs @ 12.85 hrs, Volume=	0.029 af, Atten= 83%, Lag= 45.6 min
Discarded =	0.02 cfs @ 9.65 hrs, Volume=	0.021 af
Primary =	0.13 cfs @ 12.85 hrs, Volume=	0.008 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 150.67' @ 12.85 hrs Surf.Area= 0.011 ac Storage= 0.034 af Plug-Flow detention time= 161.2 min calculated for 0.029 af (47% of inflow) Center-of-Mass det. time= 73.4 min ( 840.8 - 767.4 )

#	Invert	Avail.St	orage	Storage Description
1	146.00'	0.0	)22 af	10.00'D x 4.00'H Vertical Cone/Cylinder x 3 Inside #2
2	145.00'	0.0	)13 af	14.00'D x 5.00'H Vertical Cone/Cylinder × 3
				0.053 af Overall - 0.022 af Embedded = 0.031 af x 40.0% Voids
		0.0	34 af	Total Available Storage
#	Routing	Invert	Outle	t Devices
1	Discarded	0.00'	0.002	780 fpm Exfiltration over entire Surface area
2	Primary	150.50'	12.0"	x 115.0' long Culvert RCP, sq.cut end projecting, Ke= 0.500
	•		Outle	t Invert= 149 00' S= 0.0130 '/' n= 0.012 Cc= 0.900

**Discarded OutFlow** Max=0.02 cfs @ 9.65 hrs HW=145.07' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.02 cfs)

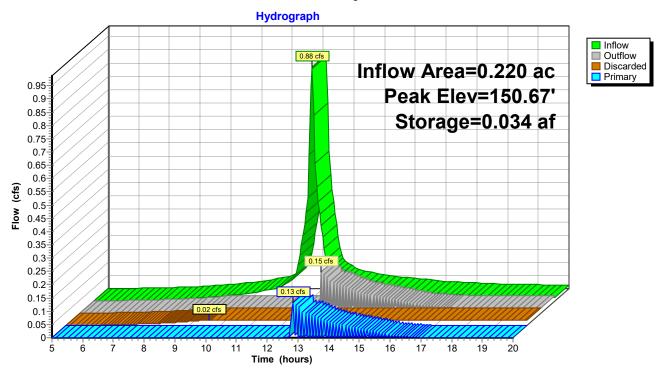
Primary OutFlow Max=0.13 cfs @ 12.85 hrs HW=150.67' (Free Discharge)

2=Culvert (Inlet Controls 0.13 cfs @ 1.4 fps)

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# Pond 5P: Drywells



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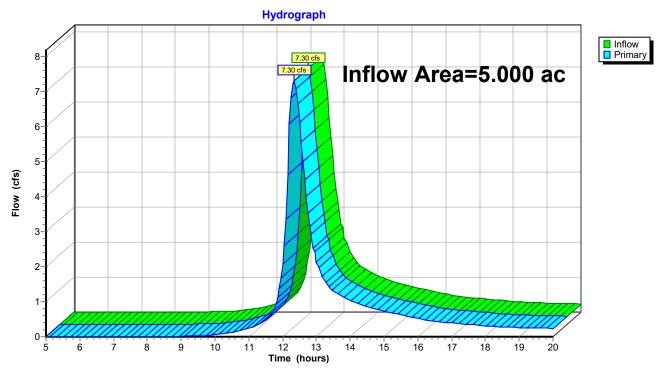
## Link 1L: ODP-1 (Culvert)

Inflow Area = 5.000 ac, Inflow Depth = 1.93" for 10-YR event Inflow = 7.30 cfs @ 12.34 hrs, Volume= 0.804 af

Primary = 7.30 cfs @ 12.34 hrs, Volume= 0.804 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

# Link 1L: ODP-1 (Culvert)



Type III 24-hr 100-YR Rainfall=9.00"

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Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1 Post: Area to Infiltration Trench

Runoff Area=0.170 ac Runoff Depth=6.79"

Tc=6.0 min CN=85 Runoff=1.34 cfs 0.096 af

**Subcatchment 2 Post: Area Direct to Dry Detention** 

Runoff Area=0.250 ac Runoff Depth=7.24"

Tc=6.0 min CN=89 Runoff=2.05 cfs 0.151 af

**Subcatchment 3 Post: Remaining Subcatchment to ODP-1** 

Runoff Area=4.360 ac Runoff Depth=5.20"

Flow Length=620' Tc=24.5 min CN=72 Runoff=17.59 cfs 1.889 af

**Subcatchment 4 Post: Area to Drywells** 

Runoff Area=0.220 ac Runoff Depth=7.13"

Tc=6.0 min CN=88 Runoff=1.79 cfs 0.131 af

Pond 1P: Infiltration Trench

Peak Elev=149.75' Storage=394 cf Inflow=1.34 cfs 0.096 af

Discarded=0.03 cfs 0.028 af Primary=1.27 cfs 0.064 af Outflow=1.30 cfs 0.092 af

Pond 2P: Dry Detention Basin

Peak Elev=149.73' Storage=1,273 cf Inflow=3.30 cfs 0.214 af

Outflow=3.18 cfs 0.212 af

Pond 5P: Drywells

Peak Elev=151.27' Storage=0.034 af Inflow=1.79 cfs 0.131 af

Discarded=0.02 cfs 0.025 af Primary=1.98 cfs 0.072 af Outflow=2.00 cfs 0.097 af

Link 1L: ODP-1 (Culvert)

Inflow=19.91 cfs 2.172 af

Primary=19.91 cfs 2.172 af

Total Runoff Area = 5.000 ac Runoff Volume = 2.266 af Average Runoff Depth = 5.44"

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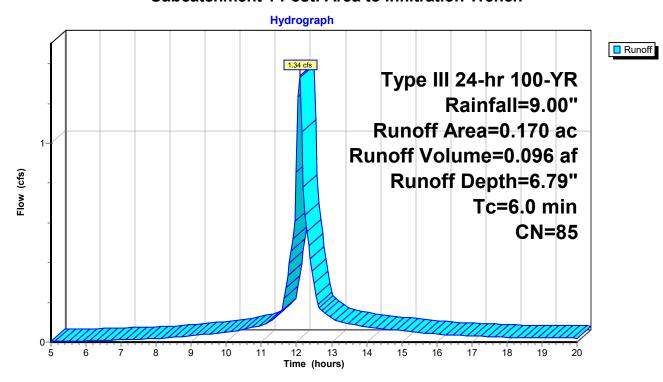
#### **Subcatchment 1 Post: Area to Infiltration Trench**

Runoff = 1.34 cfs @ 12.09 hrs, Volume= 0.096 af, Depth= 6.79"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-YR Rainfall=9.00"

Area	(ac)	CN	Desc	cription		
0.	.040	98	Pave	ed parking	& roofs	
0.	.030	65	Woo	ds/grass c	omb., Fair,	r, HSG B
0.	.100	85	Grav	el roads, l	HSG B	
0.	.170	85	Weig	ghted Aver	age	
Tc	Leng	th	Slope	Velocity	Capacity	Description
(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)	·
6.0						Direct Entry,

## **Subcatchment 1 Post: Area to Infiltration Trench**



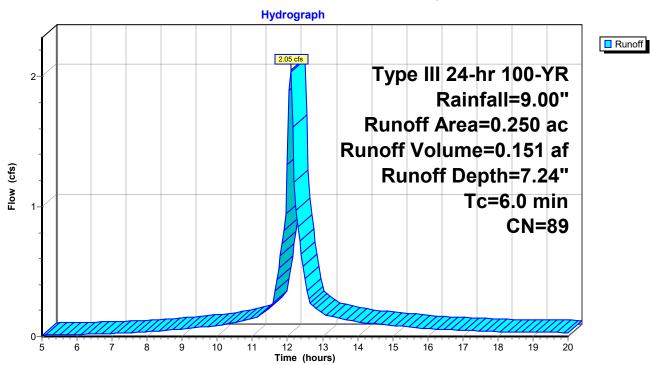
#### **Subcatchment 2 Post: Area Direct to Dry Detention**

Runoff = 2.05 cfs @ 12.09 hrs, Volume= 0.151 af, Depth= 7.24"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-YR Rainfall=9.00"

Area	(ac)	CN	Desc	ription		
0.	170	98				
0.	.080	69	50-7	5% Grass	cover, Fair	r, HSG B
0.	250	89	Weig	ghted Aver	age	
Tc (min)	Lengt		Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	(100	·- <i>)</i>	(1011)	(1000)	(010)	Direct Entry,

# **Subcatchment 2 Post: Area Direct to Dry Detention**



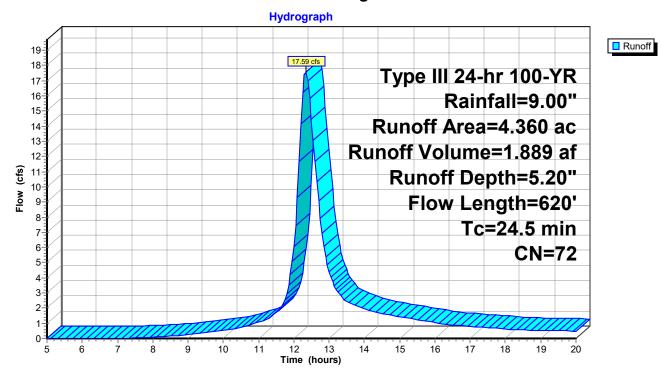
## **Subcatchment 3 Post: Remaining Subcatchment to ODP-1**

Runoff = 17.59 cfs @ 12.34 hrs, Volume= 1.889 af, Depth= 5.20"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-YR Rainfall=9.00"

Are	ea (	ac) C	N Desc	cription			
	1.3	320 9	98 Pave	ed roads w	/curbs & se	ewers	
	0.2	220	59 50-7	5% Grass	cover, Fair	r, HSG B	
	2.	750 (	30 Woo	ds, Fair, F	ISG B		
	0.0	070	31 >75°	% Grass co	over, Good	, HSG B	
	4.3	360	72 Wei	ghted Aver	age		
7	Тс	Length	Slope	Velocity	Capacity	Description	
(mi	n)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
7	.2	100	0.0400	0.2		Sheet Flow,	
						Grass: Short n= 0.150 P2= 3.40"	
17	'.3	520	0.0100	0.5		Shallow Concentrated Flow,	
						Woodland Kv= 5.0 fps	
24	.5	620	Total				

## **Subcatchment 3 Post: Remaining Subcatchment to ODP-1**



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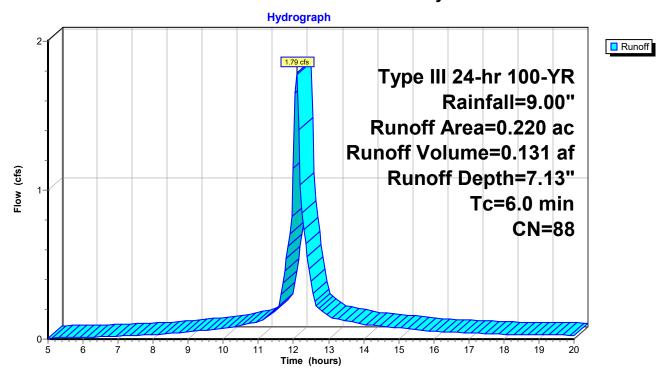
## **Subcatchment 4 Post: Area to Drywells**

Runoff = 1.79 cfs @ 12.09 hrs, Volume= 0.131 af, Depth= 7.13"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100-YR Rainfall=9.00"

Area	(ac)	CN	Desc	cription		
	.120	98				
C	0.060	85	Grav	el roads, l	HSG B	
	0.040	61	>759	% Grass co	over, Good	I, HSG B
	).220	88	Wei	ghted Aver	age	
Тс	Leng	gth	Slope	Velocity	Capacity	Description
(min)	(fe	et)	(ft/ft)	(ft/sec)	(cfs)	
6.0						Direct Entry,

#### **Subcatchment 4 Post: Area to Drywells**



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#### **Pond 1P: Infiltration Trench**

Inflow Area =	0.170 ac, Inflow Depth = 6.79"	for 100-YR event
Inflow =	1.34 cfs @ 12.09 hrs, Volume=	0.096 af
Outflow =	1.30 cfs @ 12.11 hrs, Volume=	0.092 af, Atten= 3%, Lag= 1.3 min
Discarded =	0.03 cfs @ 8.70 hrs, Volume=	0.028 af
Primary =	1.27 cfs @ 12.11 hrs, Volume=	0.064 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 149.75' @ 12.11 hrs Surf.Area= 564 sf Storage= 394 cf Plug-Flow detention time= 32.7 min calculated for 0.092 af (95% of inflow)

Center-of-Mass det. time= 14.4 min (772.0 - 757.5)

#_	Invert	Avail.S	orage Storage D	Description			
1	148.00'			Custom Stage Data (Prismatic) Listed below			
			1,692 CI C	Overall x 40.0% Voids			
Elev	ation	Surf.Area	Inc.Store	Cum.Store			
(	feet)	(sq-ft)	(cubic-feet)	(cubic-feet)			
14	18.00	564	0	0			
15	151.00 564		1,692	1,692			
#	Routing	Invert	Outlet Devices				
1	Discarded	0.00'	0.002780 fpm Exf	filtration over entire Surface area			
2	Primary	151.00'	3.0' long x 10.0' breadth Broad-Crested Rectangular Weir				
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60				
3	Primary	149.00'	Coef. (English) 2.49 2.56 2.70 2.69 2.68 2.69 2.67 2.64 <b>12.0" x 52.0' long Culvert</b> CPP, projecting, no headwall, Ke= 0.900				
	,			8.80' S= 0.0038 '/' n= 0.012 Cc= 0.900			

**Discarded OutFlow** Max=0.03 cfs @ 8.70 hrs HW=148.03' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.03 cfs)

Primary OutFlow Max=1.24 cfs @ 12.11 hrs HW=149.74' (Free Discharge)

-2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

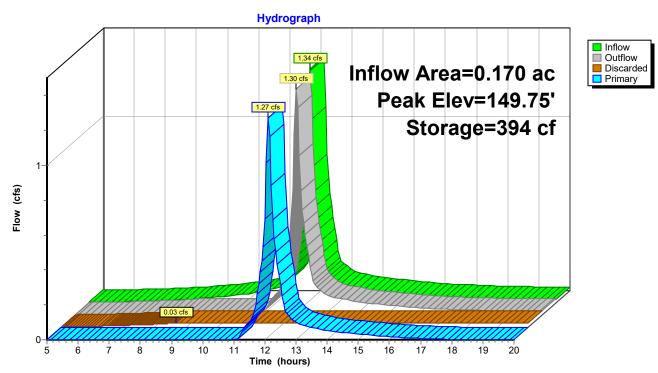
-3=Culvert (Barrel Controls 1.24 cfs @ 2.8 fps)

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#### **Pond 1P: Infiltration Trench**



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#### Pond 2P: Dry Detention Basin

Inflow Area = 0.420 ac, Inflow Depth = 6.13" for 100-YR event Inflow = 3.30 cfs @ 12.10 hrs, Volume= 0.214 af

Outflow = 3.18 cfs @ 12.12 hrs, Volume= 0.212 af, Atten= 3%, Lag= 1.3 min

Primary = 3.18 cfs @ 12.12 hrs, Volume= 0.212 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 149.73' @ 12.12 hrs Surf.Area= 1,567 sf Storage= 1,273 cf Plug-Flow detention time= 29.7 min calculated for 0.211 af (98% of inflow)

Center-of-Mass det. time= 24.2 min (775.5 - 751.3)

#	Invert	Avail.S	torage	Storage De	scription	
1	148.80'	3,	635 cf	Custom Sta	age Data (Prisma	atic) Listed below
	ation (feet)	Surf.Area (sq-ft)		Inc.Store	Cum.Store (cubic-feet)	
	18.80	900		0	0	
14	19.00	1,200		210	210	
15	50.00	1,700		1,450	1,660	
15	51.00	2,250		1,975	3,635	
#	Routing	Invert	Outlet	Devices		
1	Primary	150.10'	12.0' ld	ong x 8.0' br	eadth Broad-Cro	ested Rectangular Weir
			,	,		1.00 1.20 1.40 1.60 1.80 2.00 2.50
					50 5.00 5.50	
						9 2.68 2.68 2.66 2.64 2.64 2.64 2.65
					68 2.70 2.74	
2	Primary	148.80'			rate C= 0.600	
3	Primary	149.50'				rested Rectangular Weir
						1.00 1.20 1.40 1.60
			Coef. (	English) 2.4	9 2.56 2.70 2.6	9 2.68 2.69 2.67 2.64

**Primary OutFlow** Max=3.09 cfs @ 12.12 hrs HW=149.73' (Free Discharge)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

**—2=Orifice/Grate** (Orifice Controls 0.37 cfs @ 4.2 fps)

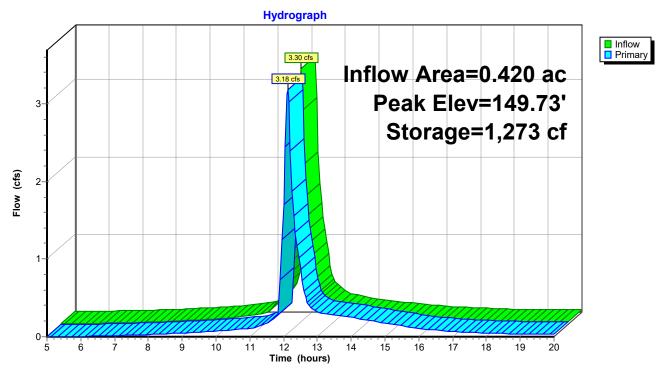
—3=Broad-Crested Rectangular Weir (Weir Controls 2.72 cfs @ 1.2 fps)

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# Pond 2P: Dry Detention Basin



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## Pond 5P: Drywells

Inflow Area =	0.220 ac, Inflow Depth = 7.13"	for 100-YR event
Inflow =	1.79 cfs @ 12.09 hrs, Volume=	0.131 af
Outflow =	2.00 cfs @ 12.06 hrs, Volume=	0.097 af, Atten= 0%, Lag= 0.0 min
Discarded =	0.02 cfs @ 7.35 hrs, Volume=	0.025 af
Primary =	1.98 cfs @ 12.06 hrs, Volume=	0.072 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 151.27' @ 12.05 hrs Surf.Area= 0.011 ac Storage= 0.034 af Plug-Flow detention time= 93.0 min calculated for 0.096 af (74% of inflow) Center-of-Mass det. time= 30.5 min (782.1 - 751.6)

#	Invert	Avail.Storage		Storage Description
1	146.00'	0.022 af		10.00'D x 4.00'H Vertical Cone/Cylinder x 3 Inside #2
2	145.00'	0.013 af		14.00'D x 5.00'H Vertical Cone/Cylinder x 3
				0.053 af Overall - 0.022 af Embedded = 0.031 af x 40.0% Voids
		0.034 af Total Available Storage		
#	Routing	Invert	Outlet Devices	
1	Discarded	0.00'	0.002780 fpm Exfiltration over entire Surface area	
2	Primary	150.50'	12.0" x 115.0' long Culvert RCP, sq.cut end projecting, Ke= 0.500	
			Outle	t Invert= 149 00' S= 0 0130 '/' n= 0 012 Cc= 0 900

**Discarded OutFlow** Max=0.02 cfs @ 7.35 hrs HW=145.07' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.02 cfs)

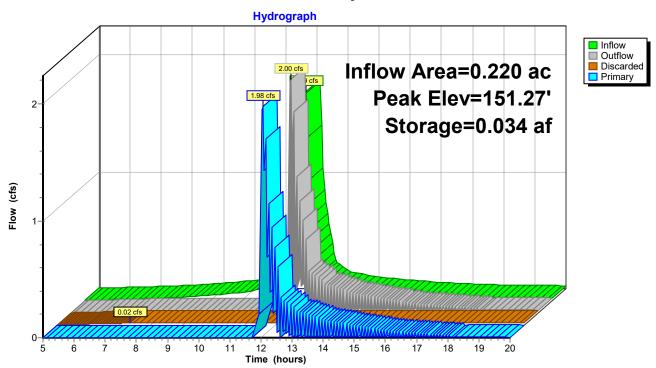
Primary OutFlow Max=1.85 cfs @ 12.06 hrs HW=151.25' (Free Discharge) —2=Culvert (Inlet Controls 1.85 cfs @ 2.9 fps)

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# Pond 5P: Drywells



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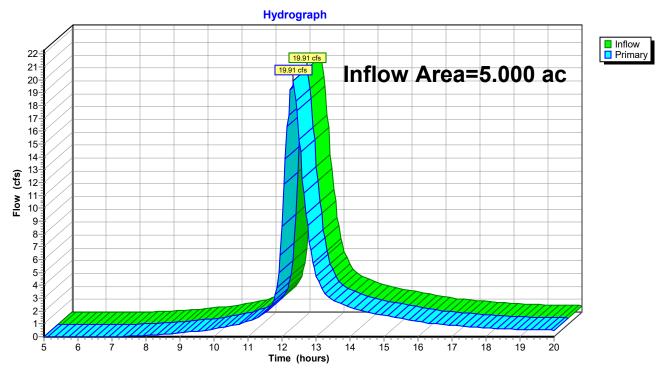
## Link 1L: ODP-1 (Culvert)

Inflow Area = 5.000 ac, Inflow Depth = 5.21" for 100-YR event 19.91 cfs @ 12.34 hrs, Volume= 2.172 af

Primary = 19.91 cfs @ 12.34 hrs, Volume= 2.172 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

# Link 1L: ODP-1 (Culvert)



OFF-SITE DISCHARGE POINT (EXISTING 12" RCP)

# CULVERT CAPACITY 12-INCH DIAMETER PIPE

