GENERAL NOTES

1. THE WORK SHOWN ON THESE DRAWINGS HAS BEEN DESIGNED IN ACCORDANCE WITH THE STRUCTURAL REQUIREMENTS OF THE OFFICIAL COMPILATION OF CODES, RULES AND REGULATIONS OF THE STATE OF NEW YORK, JANUARY, 2023.

2. THE STRUCTURAL COMPONENTS HAVE BEEN DESIGNED FOR THE FOLLOWING LIVE LOADS:

   A: FLOOR LIVE LOADS:

   I. 60 LB. LIVE LOAD @ BALCONIES
   10 LB. LIVE @ ROOMS
   40 LB. LIVE LOAD @ ATTICS W/O STORAGE
   40 LB. LIVE @ ROOMS
   40 LB. LIVE @ SLEEPING ROOMS
   40 LB. LIVE @ STAIRS
   200 LB. IN ANY DIRECTION @ HANDRAILS

   II. MECHANICAL 100 PSF OR ACTUAL WEIGHT OF EQUIPMENT.

   B: ROOF SNOW LOAD:

   I. GROUND SNOW LOAD, Pg 30 PSF
   II. EXPOSURE FACTOR, Ce 1.0
   III.THERMAL FACTOR, CT 1.0
   IV. IMPOSTANCE FACTOR, Is 1.0
   V. FLAT ROOF SNOW LOAD, Pf 39 PSF
   UNBALANCE, DRIFT AND SLIDING SNOW

   WIND LOAD:
   I. WIND LOADS HAVE BEEN DETERMINED BASED ON SECTION 1609.6. SIMPLIFIED PROVISIONS FOR LOW RISE BUILDINGS.
   II. BASIC WIND SPEED (3-SECOND GUST) Special Wind Region
   III. IMPOSTANCE FACTOR, Iw 1.0
   IV. EXPOSURE B
   V. INTERNAL PRESSURE COEFFICIENT 0.18

3. ALL STRUCTURAL WORK SHOWN OR SPECIFIED ON THESE DRAWINGS IS SUBJECT TO REVIEW BY THE STRUCTURAL ENGINEER OF RECORD OF RECORD. ASPECTS OF THE WORK.. FOUND TO BE DETECTIVE BECAUSE IT DOES NOT MEET THE REQUIREMENTS SHOWN OR SPECIFIED SHALL BE CORRECTED AS DIRECTED BY THE ENGINEER.

4. THIS WORK HAS BEEN DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE CONSTRUCTION HAS BEEN COMPLETED. THE STABILITY OF THE STRUCURAL PRIOR TO COMPLETION IS SOLELY THE RESPONSIBILITY OF THE CONTRACTOR. THIS RESPONSIBILITY EXTENDS TO ALL ASPECTS OF THE CONSTRUCTION ACTIVITY INCLUDING, BUT NOT LIMITED, JOBSITE SAFETY, ERECTION METHODS, ERECTION SEQUENCE, TEMPORARY BRACING, AND SHORING, USE OF EQUIPMENT AND SIMILAR CONSTRUCTION PROCEDURES. REVIEW OF THE CONSTRUCTION BY THE ENGINEER IS FOR THE CONFORMANCE WITH THE DESIGN ASPECTS ONLY, NOT THE REVIEW THE CONTRACTORS CONSTRUCTION PROCEDURES. LACK OF COMMENT ON THE PART OF THE ENGINEER WITH REGARD TO CONSTRUCTION PROCEDURES IS NOT TO BE INTERPRETED AS APPROVAL OF THESE PROCEDURES.

FOUNDATION NOTES:

1. THE NEW FOUNDATIONS HAVE BEEN DESIGNED TO REST ON INORGANIC UNDISTUBED SOIL HAVING A PRESUMPTIVE BEARING VALVE OF 3,000 PSF EXPECTED TO BE AT THE BOTTOM OF THE EXCAVATION. THE ENGINEER WILL BE OTIFIED IF THE SOIL OF QUESTIONABLE CAPACITY IS ENCOUNTERED DURING EXCAVATION.

2. WITHIN THE PERIMETER OF THE PROPOSED STRUCTURE STRIP THE GROUND SURFACE OF ALL TOPSOIL ORGANIC AND FIL MATERIAL.

3. THE BOTTOM OF EXTERIOR FOOTING NOT ON SOLID ROCK SHALL BE AT LEAST 4'-0" BELOW FINISHED GRADE. FOOTINGS ON LEDGE SHALL REST ON BROOM CLEAN SOLID ROCK. IF THE SLOPE OF ROCK SURFACE EXCEEDS 1 ON 6, THE FOOTING SHALL BE DOWELED TO THE LEDGE WITH 3/4" STEEL RODS DRILLED 10 INCHES INTO THE ROCK SURFACE AT 2 FEET ON CENTER.

4. IN AREAS REQUIRING FILL, THE FILL MATERIAL SHALL BE A UNIFORMLY GRADED MIXTURE OF SAND AND GRAVEL WEIGHING NO LESS THAN 120 PCF DRY DENSITY AFTER COMPACTION IN PLACE. THIS MIXTURE SHALL BE UNIFORMLY GRADED HAVING NO STONE GREATER THAN 3 INCHES IN ANY ONE DIMENSION AND WITH LESS THAN 1 % BY WEIGHT, PASSING A # 100 SIEVE. THE FILL SHALL BE PLACED ON MAXIMUM LIFTS OF 6 INHES BEFORE COMPACTION. EACH LIFT SHALL BE COMPACTED WITH APPROPRIATE EQUIPMENT TO A MINIMUM OF 9 % OF ITS MAXIMUM DENSITY AT OR NEAR OPTIMUM MOISTURE. A SOIL TESTING LAB, HIRED BY THE OWNER, SHALL TEST THE MATERIAL BEFORE AND AFTER THE COMPACTION FOR CONFORMANCE WITH THIS SECIFICATION. NO LIFTS SHALL BE PLACED WHEN WEATHER CONDITIONS ARE SUCH THAT THE MOISTURE CONTENT OF THE FILL CANNOT BE PROPERLY CONTROLLED.

5. IN PLACING AND COMPACTING FILL AND BACKFILL MATERIAL, DO NOT DAMAGE NOR DISPLACE CONCRETE WORK ALREADY IN PLACE BY CONTACT FROM COMPACTION MACHINERY, BY SUBJECTING IT TO OVERTURNING FROM HEAVY COMPACTING LOADINGS, OR ANY OTHER CAUSE. BRING FILL AGAINST SUCH CONCRETE AT THE SAME RATE AS THE REMAINDER FILL. COMPACT UNIFORMLY ON BOTH SIDES USING HAND TAMPERS. IN BASEMENT / CRAWL AREAS DO NOT BACKFILL AGAINST WALLS UNTIL THE FLOOR OR ROOF DECK BEARING ON THE WALLS HAS BEEN INSTALLED AND FULLY ATTACHED TO THE TOP OF THE FOUNDATION WALL.

6. THE SLAB-ON-GRADE SUB-BASE SHALL BE CRUSHED STONE PASING A 2 INCH SIEVE AND RETAIN ON A 1/4" INCH SIEVE.

7. THE CONTRACTOR SHALL BE RESPONSIBLE FOR LIMITING POURS TO MINIMIZE SHRINKAGE CRACKING. IN GENERAL, WALLS SHALL NOT BE POURED IN CONTINUOUS LENGTHS EXCEEDING 30 FEET AND SLABS NOT EXCEEDING 20 FEET WITHOUT CONTROL JOINTS.

8. MINIMUM ANCHOR BOLT REQUIREMENTS FOR ATTACHMENT OF SUPERSTRUCTURE SHALL BE AS FOLLOWS:

   CRAWL SPACES AND SLAB ON GRADE: 1/2" DIAMETER AT 6'-0" O.C.

   FULL HEIGHT BASEMENT: 3/4" DIAMTER AT 4'-0" O.C. MAX. SPACING

   EMBED ANCHOR BOLTS A MINIMUM OF 15 INCHES INTO MASONRY, 7 INCHES INTO CAST CONCRETE. INSTALL BOLTS WITHIN 12 INCHES OF CORNERS ON ALL EXTERIOR WALLS.

9. SIZES AND LOCATIONS OF ALL REQUIRED EMBEDDED ITEMS FOR ALL TRADES SUCH AS ANCHR BOLTS, PIPING SLEEVES, HOLDOWN ACHORS, ETC. MAXIMUM SPACING SHALL BE COORDINATED BY CONTRACTOR WITH OTHER TRADES.

10. FOOTING DRAIN INVERTS ARE TO BE SET A MINIMUM OF 2 INCHES ABOVE THE BOTTOM OF ADJACENT FOOTINGS.

11. THE FOUDATIONS HAVE BEEN DESIGNED TO REST ON INORGANIC, UNDISTURBED SOIL HAVING A PRESUMPTIVE BEARING CAPACITY OF 3,000 PSF. THE BEARING STRAT IS ANTICIPATED TO VARY BETWEEN SOIL AND ROCK BEARING. WHERE THE ROCK IS EXCAVATED, IT MAY BE NECESSARY TO OVER EXCAVATE AND PROVIDE A LAYER OF STRUCTURAL FILL AS REQUIRED TO CREATE A LEVEL SURFACE FOR FOOTINGS, WHERE FOOTING TRANSITION BETWEEN ROCK AND SOIL BEARING, CREATE A TRANSITION ZONE. SEE TYPICAL DETAIL. ALL BEARING STRATA SHALL BE REVIEWED BY THE BUILDING OFFICIAL PRIOR TO PLACING CONCRETE.

CONCRETE NOTES

1. STRUCTURAL CONCRETE WORK SHALL CONFORM TO ALL THE REQUIREMENTS OF A.C.I. 301-95, "SPECIFICATIONS FOR STRUCTURAL CONCRETE IN BUILDINGS" IN ITS ENTIRETY. CERTAIN PORTIONS OF THIS SPECIFICATION ARE PRESENTED HERE ONLY FOR CLARIFICATION AND THE CONTRACTOR'S CONVENIENCE AND ARE NOT INTENDED TO REPLACE OR ADMEND THIS SPECIFICATION.

2. CONCRETE SHALL BE NORMAL WEIGHT, DEVELOP A MINIMUM 28 DAY STRENGTH, Pc AND HAVE A MAXIMUM WATER/CEMENT RATIO (W/C0 AS FOLLOWS:

LOCATION	STRENGTH
FOOTINGS	3000 PSI
SLABS	4000 PSI
WALLS	3000 PSI

3. NO ADMIXTURES ARE PERMITTED WITHOUT THE ENGINEERS WRITTEN PERMISSION OTHER THAN ENTRAINED AIR. CONCRETE EXPOSED TO THE WEATHER, SUCH AS THAT USED IN FOUNDATION WALLS, SHALL CONTAIN 5 ENTRAINED AIR , INTERIOR SLABS SHALL CONTAIN NO MORE THAN 2 ENTRAINED AIR.

4. CEMENT SHALL BE TYPE 1 OR TYPE 2, CONFORMING TO ASTM C 150.

5. AGGREGATES SHALL CONFORM TO ASTM C 33, MAXIMUM COARSE AGGREGATE SIZE TO BE 3/4".

6. REINFORCING STEEL SHALL CONFORM TO ASTM A 615, GRADE 60.

7. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 185 WITH MINIMUM YIELD STRENGTH OF 75 KSI. LAP ONE MESH SIZE AT SIDES AND ENDS, AND WIRE TOGETHER.

8. NO WELDING OF REINFORCING WILL BE PERMITTED.

9. CONCRETE FORMWORK SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 5, ACI-301 - 95.

10. FABRICATION AND PLACEMENT OF REINFORCEMENT SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 5, ACI-301 - 95.

11. CONSTRUCTION JOINTS AND EMBEDDED ITEMS, SUCH AS PIPING SLEEVES, SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 6, ACI-301-95.

12. THE PRODUCTION OF CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 7 ACI-301 - 95.

13. THE CONVEYANCE, PLACEMENT AND PROTECTION OF THE CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 8, ACI-301-95. MECHANICAL VIBRATORS ARE TO BE USED TO CONSOLIDATE THE FRESHLY CAST CONCRETE AROUND THE REINFORCING AND AGAINST FORM SURFACES AND TO PREVENT THE FORMATION OF AIR OR STONE POCKETS, HONEYCOMBING, PITTING OR PLANES OF WEAKNESS. HOWEVER, CARE MUST BE USED TO AVOID OVERVIBRATION THAT CAN LEAD TO AGGREGATE SEGREGATION.

14. THE INSTALLATION OF SLABS SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 11, ACI-301 - 95. INTERIOR FINISH SLAB SURFACES ARE TO HAVE A CLASS A STEEL TROWEL FINISH. SURFACES OF SLABS FORMING THE SUBSTRATE FOR MUD JOBS ARE TO HAVE A CLASS C SCRATCHED SURFACE. EXTERIOR SLAB SURFACES ARE TO HAVE A CLASS B TOLERANCE WITH THE FINISH AS SPECIFIED ON THE ARCHITECTURAL DRAWINGS.

15. THE CURING AND PROTECTION OF CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF CHAPTER 12 ACI-301 - 95. CONCRETE SLABS SHALL BE PROTECTED FROM LOSS OF SURFACE MOISTURE FOR NOT LESS THAN 7 DAYS USING A CURING COMPOUND CONFORMING TO ASTM C 309-81 OR CONSTANTLY WETTED BURLAP. IF COLD WEATHER CONCRETING CONDITIONS EXIST AS DEFINED BY A PERIOD OF MORE THAN THREE DAYS WHEN THE AVERAGE OUTDOOR TEMPERATURE, (HIGH + LOW)/2, IS LESS THAN 40° F, THE PROCEDURES OUTLINES IN ACI-306.1-87 STANDARD SPECIFICATION FOR "COLD WEATHER CONCRETING" SHALL BE UTILIZED.

MASONRY NOTES:

1. CONCRETE BLOCK SHALL BE NORMAL WEIGHT, HOLLOW LOAD BEARING MASONRY UNITS CONFORMING TO ASTM C-90, GRADE N-1, EXCEPT THAT THEY MUST HAVE A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 2,000 PSI ON THE NET AREA OF THE UNITS.

2. MORTAR SHALL BE TYPE S AND CONFORM TO ASTM C-270. MORTAR SHALL BE ONE PART PORTLAND CEMENT, 0.25 TO 0.50 PARTS HYDRATED LIME, AND 3.5 PARTS SAND.

3. COARSE GROUT USED IN PILASTERS AND WALLS SHALL CONFORM TO ASTM C-476. COARSE GROUT SHALL BE A MIXTURE OF ONE PART PORTLAND CEMENT, 3 PARTS SAND, AND 1.5 PARTS PEA GRAVEL.

4. CEMENT USED IN MORTAR AND GROUT SHALL CONFORM TO ASTM C-150. MASONRY CEMENT SHALL NOT BE USED.

5. REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60.

6. HORIZONTAL JOINT REINFORCEMENT SHALL BE DUR-O-WALL LADDER TRUSS TYPE. OR EQUAL. PROVIDE JOINT REINFORCING AT EVERY OTHER COURSE.

DIMENSIONED WOOD FRAMING NOTES:

1. THE STRUCTURAL WOOD STRESS GRADE STAMPED LUMBER SHALL BE GRADED AS FOLLOWS:

   JOISTS AND RAFTERS: DOUGLAS FIR-LARCH OR DOUGLAS FIR-LARCH (NORTH), NO. 2.
   Fb (BASE) = 850 PSI,
   E = 1,600,000 PSI

   STUDS: DOUGLAS FIR-LARCH OR DOUGLAS FIR-LARCH (NORTH), STUD GRADE.
   Fb (BASE) = 650 PSI,
   E = 1,400,000 PSI

2. THE DESIGN OF THE DIMENSIONAL LUMBER MEMBERS AND THEIR CONNECTIONS IS BASED ON THE LUMBER HAVING A MOISTURE CONTENT AT THE TIME OF INSTALLATION OF 19 PERCENT OR LESS.

3. ANY OF THESE MATERIALS MAY BE USED INTERCHANGEABLY FOR MEMBERS THAT ARE NOTED ON PLAN SINGULARLY AS LVL. MEMBER SIZES SHOWN ON PLAN (WIDTH x DEPTH) MAY BE CONSTRUCTED OF MULTIPLE PLIES OF THE SPECIFIED DEPTH, FASTENED TOGETHER BY NAILING OR BOLTING AS REQUIRED. MEMBER SIZES FOLLOWED ONLY BY PSL MUST BE INSTALLED AS A SOLID MEMBER, NOT BUILT-UP.

4. MEMBERS MAY NOT BE BORED OR NOTCHED WITHOUT WRITTEN PERMISSION FROM THE ENGINEER. SEE THIS SHEET.

GENERAL WOOD NOTES

1. WOOD DESIGN IS BASED ON THE NDS "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION".

2. STUD BEARING WALLS, SHEAR WALLS, ROOF AND FLOOR DECKS SHALL BE FRAMED WITH THE MEMBER SIZES AND/OR TYPES AT THE SPACINGS SHOWN ON PLAN. THE CONTRACTOR SHALL COORDINATE LOCATIONS OF ALL PLUMBING PIPING, HVAC DUCTING AND RECESSED LIGHTING FIXTURES, ETC. PRIOR TO LAYOUT TO MINIMIZE INTERFERENCE THAT MAY REQUIRE THE ALTERING OR STRENGTHENING OF INSTALLED FRAMING.

3. ALL WOOD FRAMING IS TO BE STORED ON SITE ABOVE THE GROUND ON "STICKERS" INDOORS OR UNDER TARPS WITH ADEQUATE CLEARANCES TO ALLOW AIR CIRCULATION.

4. WALLS SHALL BE INSTALLED STRAIGHT AND PLUMB, FLOORS SHALL BE INSTALLED LEVEL AT THE PROPER ELEVATION. ROOF SHALL BE INSTALLED AT THE PITCHES INDICATED ON THE ARCHITECTURAL DRAWINGS.

5. JOISTS AND RAFTERS SHALL BE INSTALLED DIRECTLY OVER BEARING STUDS UNLESS OTHERWISE DETAILED.

6. JOISTS SHALL BE SUPPORTED LATERALLY AT EACH SUPPORT BY FULL DEPTH SOLID BLOCKING TWO INCHES IN THICKNESS, EXCEPT WHERE JOISTS ARE SUPPORTED BY A FLUSH HEADER OR NAILED TO A RIM JOIST.

7. PROVIDE A MINIMUM OF THREE STUDS AT EACH END OF ALL FLUSH FRAMED HEADERS OR BEAM, UNLESS MORE ARE INDICATED ON PLAN. PROVIDE TWO JACK STUDS AND ONE FULL KING STUD AT EACH END OF ALL DROPPED HEADERS OR BEAMS, UNLESS MORE JACK AND KING STUDS ARE INDICATED ON PLAN.

8. FLUSH FRAMED CONNECTIONS SHALL BE MADE WITH PREFABRICATED GALVANIZED STEEL HANGERS MADE BY SIMPSON STRONG-TIE, CO. INC. OR KANT-SAG CONNECTORS BY UNITED STEEL PRODUCTS CO. OF WIDTH AND DEPTH APPROPRIATE FOR THE SUPPORTED MEMBER. INSTALL WITH THE TYPE AND QUANTITY OF FASTENERS RECOMMENDED BY APPROVED OR EQUAL BASED ON MEMBER REACTIONS SHOWN ON DRAWINGS AND/OR CONFIGURATION AND PROVIDE PRODUCT DATA TO THE ARCHITECT FOR METAL CONNECTIONS ARE REQUIRED AT THE FLUSH FRAMED CONDITIONS, ROOF RAFTERS HOLD DOWNS, AND POST CAPS AND BASES.

9. STRUCTURAL WOOD FRAMING USED IN EXTERIOR APPLICATIONS OR IN CONTACT WITH CONCRETE OR MASONRY SHALL BE SOUTHERN YELLOW PINE NO.2 OR BETTER, CCA PRESERVATIVE PRESSURE TREATED WOOD.

10. BUILT-UP MEMBERS OF THREE PILES OR LESS SHALL HAVE ADJACENT PLIES NAILD TOGETHER WITH TWO ROWS OF NAILS AT 12" O.C. (10D COMMON NAILS FOR 1-1/2" PLIES, 12D COMMON NAILS FOR 1-3/4" PLIES). BUILT-UP MEMBERS OF MORE THAN 3 PLIES SHALL BE ASSEMBLED WITH 1/2" DIAMETER THRU BOLTS AT 16" O.C. STAGGERED UP AND DOWN WITH 2 INCH CLEARANCE AT TOP AND BOTTOM EDGES.

11. FLITCH BEAMS ARE TO BE ASSEMBLED WITH 1/2" DIAMETER THRU BOLTS AT 16" O.C., STAGGERED UP AN DOWN, WITH 2 INCH CLEARANCE AT TOP AND BOTTOM EDGES. ASSEMBLE FLITCH BEAM WITH BOTTOMS OF WOOD PLIES AND STEEL PLATE FLUSH. PROVIDE 3-1/2" x 3/8" x 3-1/2" STEEL BEARING PLATES AT END OF FLITCH BEAMS.

12. EXTERIOR END WALLS OF CATHEDRAL CEILING SPACS SHALL BE FRAMED WITH STUDS RUNNING CONTINUOUSLY (NOT SPLICED) FROM FLOOR TO ROOF.

PREFABRICATED WOOD TRUSS NOTES

1. PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED, IN ACCORDANCE WITH THE CURRENT EDITION OF THE "DESIGN SPECIFICATION FOR METAL PLATE CONNECTED WOOD TRUSSES" BY THE TRUSS PLATE INSTITUTE. THE MANUFACTURER SHALL DESIGN THE TRUSSES FOR THE FOLLOWING LOADS:

   TOP CHORD LIVE LOAD = SEE DESIGN LOAD SECTION.
   TOP CHORD DEAD LOAD = 15 PSF PLUS TRUSS SELF-WEIGHT.
   BOTTOM CHORD LIVE LOAD = 125 PSF
   20 PSF
   0 PSF IN INACCESSIBLE AREAS
   BOTTOM CHORD DEAD LOADS = 10 PSF PLUS TRUSS SELF WEIGHT.

2. SPACING, AND DEPTH REQUIREMENTS SPECIFIED ON THE FRAMING PLANS SHALL NOT BE EXCEEDED. DEFLECTION OF ROOF TRUSS SHALL NOT EXCEED LIVE LOAD / 360, TOTAL LOAD/240, OR 1".

3. ALL LUMBER USED IN FABRICATION OF TRUSSES SHALL BE STRESS GRADED AND SHALL BE OF THE SPECIE, SIZE, AND GRADE SPECIFIED IN THE TRUSS DESIGN.

4. CONNECTOR PLATES SHALL BE STAMPED FROM 16, 18, OR 20 GAUGE, GRADE A, GALVANIZED STRUCTURAL STEEL. BOTH PLATE WIDTH AND PLATE LENGTH MUST EQUAL OR EXCEED THAT SPECIFIED IN THE TRUSS DESIGN.

5. TRUSSES SHALL BE FABRICATED IN ACCORDANCE WITH THE TRUSS DESIGN FROM ACCURATELY CUT WOOD MEMBERS CLAMPED IN RIGID FIXTURES DURING ASSEMBLY TO INSURE TIGHT FITTING JOINTS AND UNIFORMITY IN COMPLETED TRUSS UNITS.

6. CONNECTOR PLATES ARE TO BE PRESSED INTO THE WOOD MEMBERS ON BOTH SIDES OF THE TRUSS AT EACH JOINT SO THAT FULL PENETRATION OF THE TEETH IS OBTAINED WITHOUT CRUSHING THE OUTER SURFACE IN THE TRUSS DESIGN.

7. TRUSSES SHALL BE INSTALLED PLUMB, ADEQUATELY BRACED, IN THE PROPER ORIENTATION, AND AT THE SPACING SPECIFIED IN THE TRUSS DESIGN.

8. CUTTING OF TRUSS MEMBERS OR FIELD ALTERATION OF ANY TRUSS IS NOT PERMITTED.

9. SHOP DRAWINGS AND DESIGN CALCULATION SUBMITTALS BEARING THE CERTIFICATION OF A PROFESSIONAL ENGINEER REGISTERED IN THE JURISDICTION OF THE PROJECT SHALL BE SUBMITTED TO THE ARCHITECT FOR APPROVAL. SHOP DRAWINGS SHALL CONTAIN THE FOLLOWING INFORMATION FOR EACH TYPE AND SIZE OF TRUSS ASSEMBLY TO BE PROVIDED:DETAIL OF TRUSS:SPECIE, SIZE, GRADE AND WORKING STRESS OF LUMBER USED: CALCULATED FORCE FOR EACH MEMBER AND CONNECTOR PLATE SIZES AND ORIENTATION.

WOOD FASTENERS NOTES:

1. WOOD COMPONENTS ARE TO BE FASTENED TOGETHER AS INDICATED IN THE FOLLOWING SCHEDULE UNLESS SPECIFICALLY INDICATED OTHERWISE ON THE PLANS.

   NAILING SCHEDULE

CONNECTION	NAIL SIZE/TYPE	QUANTITY OR SPACING
STUD TO SOLE PLATE:	8D COMMON 16D COMMON	4 TOE-NAIL OR 2 END DIRECT
STUD TO CAP PLATE:	8D COMMON 16D COMMON	4 TOE-NAIL OR 2 END DIRECT
DOUBLE STUDS:	10D COMMON	9" O.C. DIRECT
CORNER STUDS:	16D COMMON	9" O.C. DIRECT
SOLE PLATE TO JOIST OR BLOCKING	16D COMMOMN	8" O.C. DIRECT
DOUBLE CAP PLATE:	10D COMMON	16" O.C. DIRECT
CAP PLATE LAPS:	10D COMMON	2 DIRECT
RIM/BOS JOIST TO A JOIST END:	16D COMMON	3 END DIRECT
ROOF RAFTER TO PLATE:	10D COMMON	4 TOE-NAIL
ROOF RAFTER TO RIDGE:	16D COMMON 10D COMMON	2 END DIRECT OR 4 TOE-NAIL
JACK RAFTER TO HIP:	10D COMMON 16D COMMON	3 TOE-IN OR 2 DIRECT
FLOOR JOISTS TO SILL OR GIRDER:	10D COMMON	4 TOE-NAIL
EDGE FLOOR JOIST TO SILL:	10D COMMON	TOE-NAIL 8" O.C.
CEILING JOIST TO PLATE:	16D COMMON	4 TOE-NAIL
CEILING JOISTS LAPS OVER PARTITIONS:	10D COMMON	4 DIRECT
CEILING JOISTS TO DCTBSTW:	10D COMMON	5 DIRECT
COLLAR BEAM TO DCTBSTW:	10D COMMON	5 DIRECT
BRIDGING TO JOISTS:	10D COMMON 8D COMMON 8D COMMON	2 DIRECT 3 TOE-NAIL 6" O.C. EDGES
PLYWOOD ROOF & WALL SHEATHING: (3/4" OR LESS) (GREATER THAN 3/4")	8D COMMON OR 6D ANNULAR 10D COMMON	6" O.C. EDGES 10" O.C. OTHER 6"O.C. EDGES AND 10" O.C. OTHER.

2. NAILS AND SPIKES SHALL CONFORM TO THE NOMINAL SIZES SPECIFIED IN FEDERAL SPECIFICATIONS FF-N-105B. PNEMATIC OR ELECTRIC POWERED HAMMERS TYPICALLY UTILIZE LIGHTER GAGE FASTENERS AND NORMALLY REQUIRE ADDITIONAL FASTENERS TO BE INSTALLED. FASTENERS SPECIFICATIONS MUST BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO INSTALLATION IF POWERED HAMMERS ARE TO BE USED.

3. LAG AND WOOD SCREWS SHALL CONFORM TO ANSI/ASME STANDARD B18.6.1-19.81.

4. ALL FASTENERS USED IN CONTACT WITH PRESERVATIVE PRESSURE TREATED WOOD SHALL BE HOT DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A153 OR STAINLESS STEEL, TYPE 316.

5. BORED LEAD HOLES FOR FASTENERS SHALL BE AS FOLLOWS:

   A. NAIL AND SPIKE LEAD HOLES ARE NOT REQUIRED UNLESS TO PREVENT SPLITTING OF WOOD. IF REQUIRED, LEAD HOLE DIAMETER SHALL NOT EXCEED 75 PERCENT OF NAIL/SPIKE DIAMETER.
   B. WOOD SCREWS - LEAD HOLE DIAMETER EQUALS 7/8 OF UNTHREADED SHANK DIAMETER IN CONNECTED WOOD PART AND 7/8 OF DIAMETER AT ROOT OF THREAD IN WOOD RECEIVING THREAD.
   C. LAG SCREWS - LEAD HOLE DIAMETER EQUALS SHANK DIAMETER FOR EXTENT OF UNTHREADED SHANK, AND 60 PERCENT OF SHANK DIAMETER FOR THREADED PORTION OF SHANK.
   D. THRU-BOLTS - LEAD HOLE DIAMETER 1/32" TO 1/16" LARGER THAN NOMINAL BOLT DIAMETER.

6. INSERT THREADED SCREW TYPE FASTENERS BY TURNING WITH SCREWDRIVER OR WRENCH. DO NOT DRIVE BY HAMMERING. FACILITATE INSTALLATION BY PLACING SOAP OR OTHER LUBRICANT ON THREADS.

7. PROVIDE STANDARD ROUND WASHERS UNDER HEADS OF ALL THROUGH BOLTS AND LAG SCREWS AND UNDER ALL NUTS UNLESS OTHERWISE INDICATED ON THE PLANS. TIGHTEN FASTENERS WITHOUT CRUSHING WOOD FIBERS UNDER WASHERS.

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Village Pancake Factory
1099 Route 9
Fishkill, NY 12524
GENERAL NOTES AND SPECIFICATIONS

REVIEWS

REVISIONS	
1	7/20/2025 SITE PLANNING BOARD REVISIONS
2	10/20/2025 SITE PLANNING BOARD REVISIONS

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Date: 6/12/25 Job: 2501

Scale: N.T.S.

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